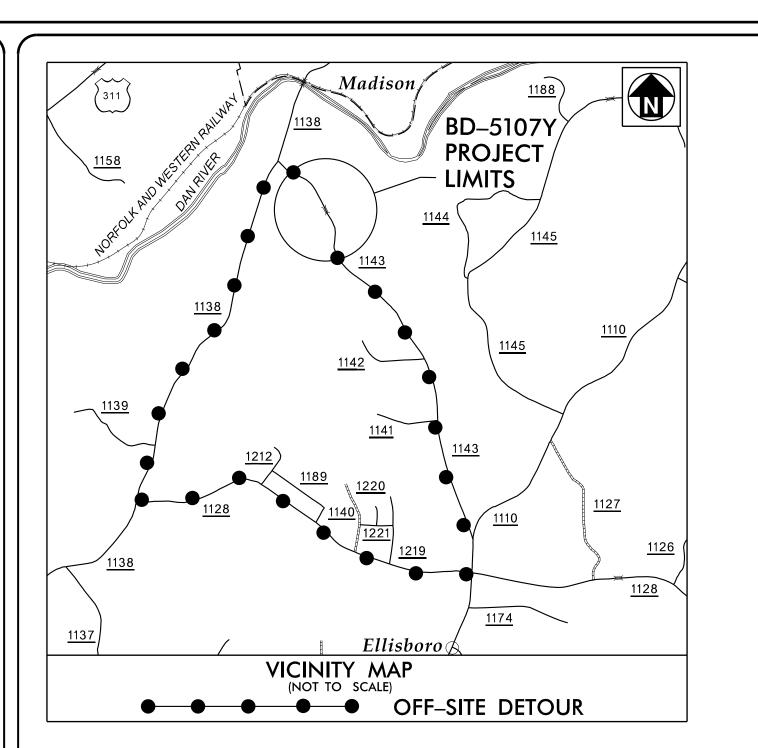
PROJECT: BD-5107

NTRACT:



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

ROCKINGHAM COUNTY

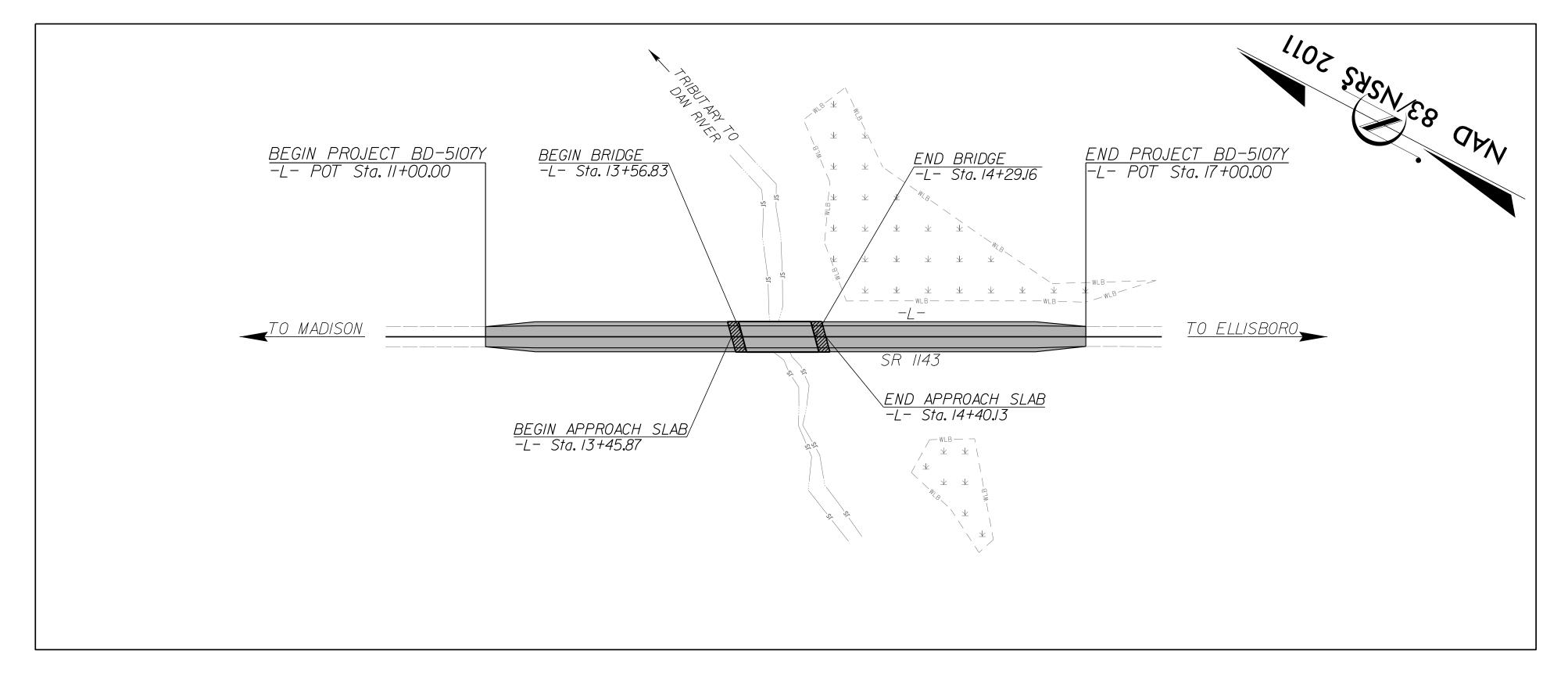
STATE PROJECT NO. F.A.PROJ.NO. DESCRIPTION

45353.1.26 BRZ-1143(10) P.E.

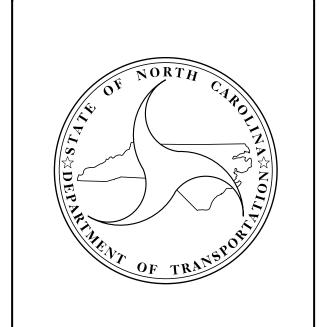
45353.1.FD26 BRZ-1143(10) CONSTRUCTION

LOCATION: BRIDGE NO.141 OVER TRIBUTARY TO DAN RIVER ON SR 1143 (RIERSON ROAD)

TYPE OF WORK: GRADING, PAVING, DRAINAGE AND STRUCTURE



*DESIGN EXCEPTION:
SAG VERTICAL CURVE K
VERTICAL SSD



DESIGN DATA

ADT 2009 = 2400

V = 50 MPH

SUB REGIONAL TIER LOCAL

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT = 0.100 MILES

LENGTH STRUCTURE TIP PROJECT = 0.014 MILES

TOTAL LENGTH TIP PROJECT = 0.114 MILES

Prepared in the Office of Hatch Mott MacDonald for

DIVISION 7

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

2012 STANDARD SPECIFICATIONS

TIM JORDAN, PE

PROJECT ENGINEER

DAVID FUH, PE

HYDRAULICS ENGINEER

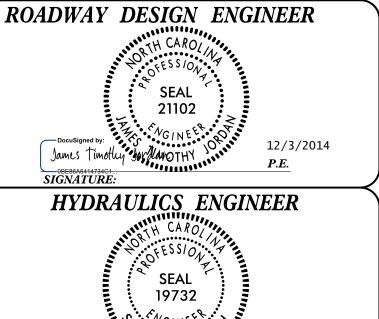
NCDOT CONTACT:

HYDRAULICS ENGINEER

SEAL
19732

DIVISION BRIDGE
PROGRAM MANAGER

Docusigned by:
Skilled Fill Chen



12/8/2014

PLANS PREPARED BY: Hatch Mott MacDonald

PO Box 700 Fuquay-Varina, NC 27526 (919) 552-2253 (919) 552-2254 (Fax) www.hatchmott.com

LICENSE NO. F-0669



GENERAL NOTES:

2012 SPECIFICATIONS EFFECTIVE: 01-17-12 REVISED: 07/30/12

GRADE LINE:

GRADING AND SURFACING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE TOWN OF MADISON, CENTURYLINK, INC., DUKE ENERGY, TIME WARNER CABLE, INC. AND PIEDMONT NATURAL GAS, INC.

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

	INDEX OF SHEETS
SHEET NUMBER	DESCRIPTION
1	TITLE SHEET
1 - A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1 -B	CONVENTIONAL SYMBOLS
2	PAVEMENT SCHEDULE AND TYPICAL SECTIONS
2-A	DETAIL FOR STRUCTURE ANCHOR UNITS
4	PLAN SHEET AND PROFILE SHEET
TMP-1 THRU TMP-3	TRAFFIC MANAGEMENT PLANS
EC-1 THRU EC-5	EROSION CONTROL PLANS
UO-1	UTILITIES BY OTHERS PLAN
X-1 THRU X-3	CROSS-SECTIONS
S-1 THRU S-17	STRUCTURE PLANS

2012 ROADWAY ENGLISH STANDARD DRAWINGS The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans: STD.NO. TITLE DIVISION 2 - EARTHWORK 200.02 Method of Clearing - Method II 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement DIVISION 3 - PIPE CULVERTS 300.01 Method of Pipe Installation 310.10 Driveway Pipe Construction DIVISION 4 - MAJOR STRUCTURES 422.11 Reinforced Bridge Approach Fills - Sub Regional Tier DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I DIVISION 6 - ASPHALT BASES AND PAVEMENTS 654.01 Pavement Repairs DIVISION 8 - INCIDENTALS 840.00 Concrete Base Pad for Drainage Structures 840.25 Anchorage for Frames - Brick or Concrete or Precast 840.29 Frames and Narrow Slot Flat Grates 840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates 840.46 Traffic Bearing Precast Drainage Structure 840.66 Drainage Structure Steps 846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter 862.01 Guardrail Placement 862.02 Guardrail Installation 876.01 Rip Rap in Channels Guide for Rip Rap at Pipe Outlets 876.02 Drainage Ditches with Class 'B' Rip Rap 876.04

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PROJECT REFERENCE SHEET NO.

BD-5107Y - ROCKINGHAM #141 1-B

Note: Not to Scale

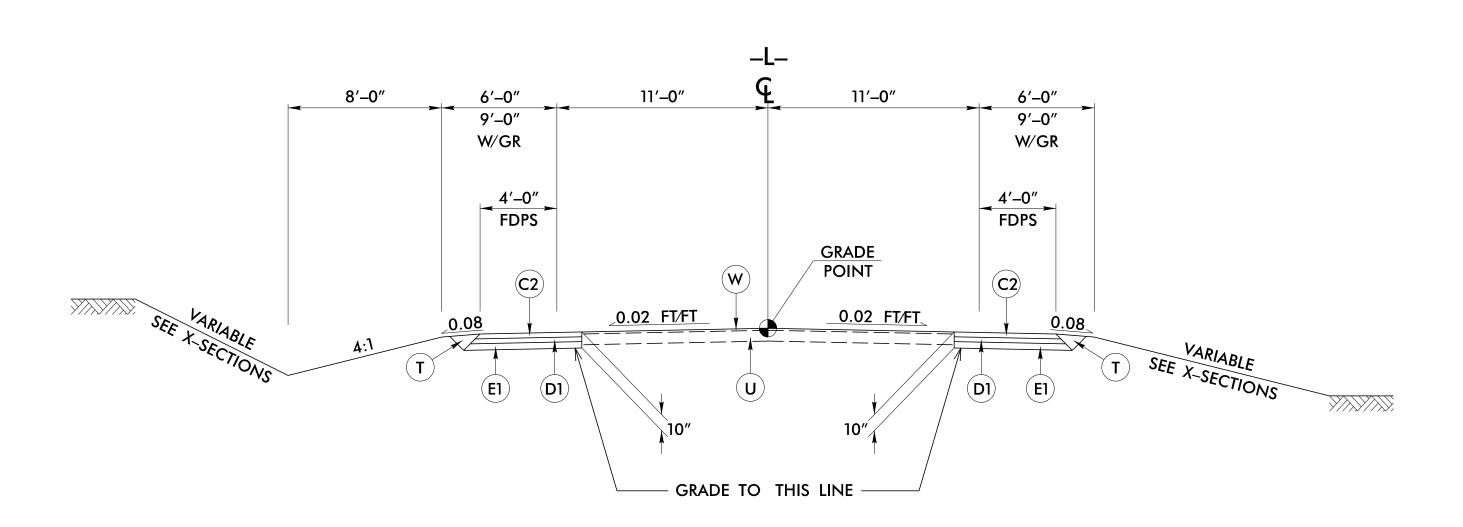
*S.U.E. = Subsurface Utility Engineering

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CONVENTIONAL PLAN SHEET SYMBOLS

WATER:

BOUNDARIES AND PROPERTY	V.					Water Manhole	W
State Line TROPERT						Water Meter	
		RAILROADS:				Water Valve	\otimes
County Line		Standard Gauge	- I ANGEODE AT LOW	EXISTING STRUCTURES:		Water Hydrant	
Township Line		RR Signal Milepost ————————————————————————————————————	CSX TRANSPORTATION MILEPOST 35	MAJOR:		Recorded U/G Water Line —————	——————————————————————————————————————
City Line		Switch		Bridge, Tunnel or Box Culvert	CONC	Designated U/G Water Line (S.U.E.*)	
Reservation Line	· ·	RR Abandoned	<i>SWITCH</i>	Bridge Wing Wall, Head Wall and End Wall	-) CONC WW (Above Ground Water Line	A/G Water
Property Line		RR Dismantled		MINOR:			
Existing Iron Pin	EIP	KK Dismanled		Head and End Wall	CONC HW	TV:	
Property Corner	×	RIGHT OF WAY:		Pipe Culvert —		TV Satellite Dish —————	$ \swarrow $
Property Monument	ECM	Baseline Control Point	-	Footbridge ————	>	TV Pedestal ————————————————————————————————————	C
Parcel/Sequence Number	(23)	Existing Right of Way Marker		Drainage Box: Catch Basin, DI or JB	СВ	TV Tower	\bigotimes
Existing Fence Line		Existing Right of Way Line		Paved Ditch Gutter		U/G TV Cable Hand Hole	H _H
Proposed Woven Wire Fence	——————————————————————————————————————	Proposed Right of Way Line	$\frac{R}{W}$	Storm Sewer Manhole	<u>(S)</u>	Recorded U/G TV Cable	TV
Proposed Chain Link Fence		Proposed Right of Way Line with	$-\frac{R}{W}$	Storm Sewer	s	Designated U/G TV Cable (S.U.E.*)	
Proposed Barbed Wire Fence		Iron Pin and Cap Marker	w –			Recorded U/G Fiber Optic Cable ———	TV FO
Existing Wetland Boundary		Proposed Right of Way Line with Concrete or Granite Marker		UTILITIES:		Designated U/G Fiber Optic Cable (S.U.E.*)	TV FO
Proposed Wetland Boundary	WLB	Existing Control of Access	(\bar{C})			Designated 6/0 Tibel Opile Cable (5.5.1.)	
Existing Endangered Animal Boundary	EAB	Proposed Control of Access		Existing Power Pole	_	GAS:	
Existing Endangered Plant Boundary	EPB	Existing Easement Line ————————————————————————————————————		Proposed Power Pole	<u> </u>	Gas Valve	\wedge
BUILDINGS AND OTHER CUI	LTURE:	Proposed Temporary Construction Easement	_	Existing Joint Use Pole	→	Gas Meter —	\Diamond
Gas Pump Vent or U/G Tank Cap		Proposed Temporary Drainage Easement		Proposed Joint Use Pole	<u>-</u>	Recorded U/G Gas Line	
Sign —		Proposed Permanent Drainage Easement —		Power Manhole	(P)	Designated U/G Gas Line (S.U.E.*)	G
Well		Proposed Permanent Utility Easement —		Power Line Tower	\boxtimes	Above Ground Gas Line	A/G Gas
Small Mine	——	,	. 52	Power Transformer	\square	Above Grooma Gas Line	
Foundation ————————————————————————————————————		ROADS AND RELATED FEATUR	RES:	U/G Power Cable Hand Hole	H _H	SANITARY SEWER:	
Area Outline		Existing Edge of Pavement		H-Frame Pole	•—•	Sanitary Sewer Manhole	(A)
Cemetery		Existing Curb		Recorded U/G Power Line		Sanitary Sewer Mannole Sanitary Sewer Cleanout ————————————————————————————————————	(+)
Building —		Proposed Slope Stakes Cut	_ <u> </u>			U/G Sanitary Sewer Line ————————————————————————————————————	···
School —		Proposed Slope Stakes Fill	F	Designated U/G Power Line (S.U.E.*)			A/G Sanitary Sew
Church —		Proposed Wheel Chair Ramp	WCR	TELEDITONIE			FSS
Dam		Existing Metal Guardrail	_ <u> </u>	TELEPHONE:		Designated SS Forced Main Line (S.U.E.*) —	
		Proposed Guardrail	_ <u> </u>	Existing Telephone Pole	-	Designated 33 Forced Main Line (3.0.E.) —	— — — FSS — —
HYDROLOGY:		Existing Cable Guiderail		Proposed Telephone Pole	- O-	AMCCELLANIE OLIC	
Stream or Body of Water ————————————————————————————————————		Proposed Cable Guiderail		Telephone Manhole	(1)	MISCELLANEOUS:	
Hydro, Pool or Reservoir ————————————————————————————————————	_ []	Equality Symbol	-	Telephone Booth	3	Utility Pole —	•
Jurisdictional Stream	Js	Pavement Removal		Telephone Pedestal ————————————————————————————————————	_	Utility Pole with Base ————————————————————————————————————	
Buffer Zone 1	BZ 1			Telephone Cell Tower	,	Utility Located Object ————————————————————————————————————	\odot
Buffer Zone 2	BZ 2	VEGETATION:		U/G Telephone Cable Hand Hole	H _H	Utility Traffic Signal Box ———————————————————————————————————	S
Flow Arrow		Single Tree	— ∷	Recorded U/G Telephone Cable	т	Utility Unknown U/G Line ——————	?UTL
Disappearing Stream ————————————————————————————————————		Single Shrub	—	Designated U/G Telephone Cable (S.U.E.*)—		U/G Tank; Water, Gas, Oil ———————————————————————————————————	
Spring —		Hedge —		Recorded U/G Telephone Conduit	тс	A/G Tank; Water, Gas, Oil ——————	
Wetland ————————————————————————————————————	<u> </u>	Woods Line		Designated U/G Telephone Conduit (S.U.E.*)	тс <u>_</u>	U/G Test Hole (S.U.E.*) ——————	
Proposed Lateral, Tail, Head Ditch ————	FLOW	Orchard —	—	Recorded U/G Fiber Optics Cable	т го	Abandoned According to Utility Records ——	AATUR
False Sump	$ \Leftrightarrow$	Vineyard —	Vineyard	Designated U/G Fiber Optics Cable (S.U.E.*)	— — — T FO— — ·	End of Information ————————————————————————————————————	E.O.I.



TYPICAL SECTION NO. 1

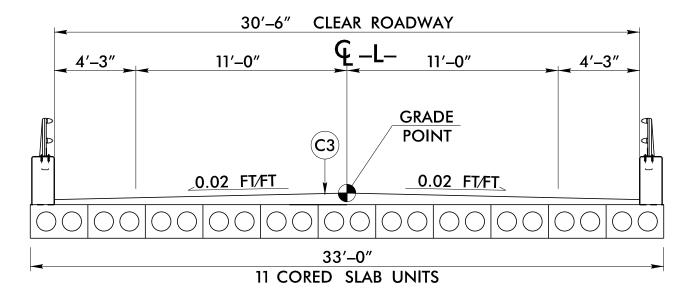
TRANSITION FROM EXISTING TO TYPICAL SECTION NO. 1: -L- STA 11+00.00 TO 11+50.00

USE TYPICAL SECTION NO. 1:

- -L- STA 11+50.00 TO 13+56.83 (BEGIN BRIDGE)
- -L- STA 14+29.16 (END BRIDGE) TO 16+50.00

TRANSITION FROM TYPICAL SECTION NO. 1 TO EXISTING:

-L- STA 16+50.00 TO 17+00.00



TYPICAL SECTION NO. 2

USE TYPICAL SECTION NO. 2:

-L- STA 13+56.83 (BEGIN BRIDGE) TO 14+29.16 (END BRIDGE)

PROJECT REFERENCE

BD-5107Y - ROCKINGHAM #141

ROADWAY DESIGN
ENGINEER

SEAL

Docusioned by:

SEAL

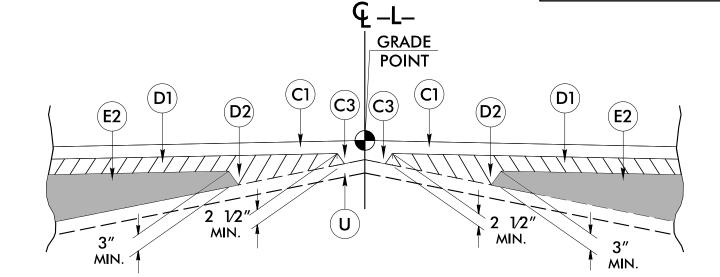
12/3/20

MACDONALD I& E, LLC
LICENSE NO. F-0669

Prepared in the
Office of:

Hatch Mott
MacDonald

PO Box 700
Fuguay-Varina, NC 27526
www.hatchmott.com



Detail Showing Method of Wedging

USE IN CONJUCTION WITH TYPICAL SECTION NO. 1

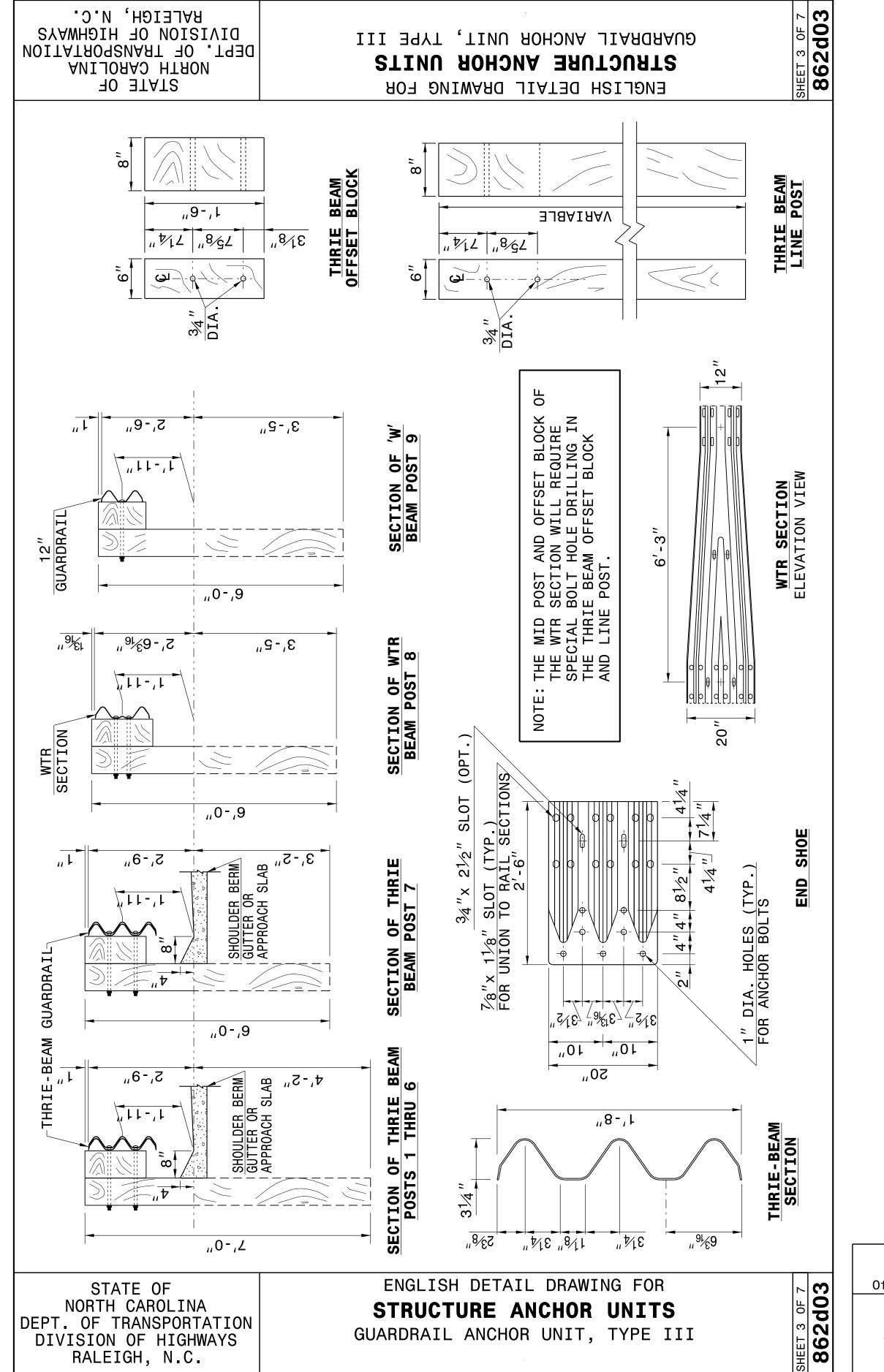
	PAVEMENT SCHEDULE
C1	PROP. APPROX. 1½" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
С3	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 1½" IN DEPTH OR GREATER THAN 2" IN DEPTH.
D1	PROP. APPROX. 3" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19.0B, AT AN AVERAGE RATE OF 342 LBS. PER SQ. YD.
D2	PROP. VAR. DEPTH ASPHALT CONCRETE BINDER COURSE. TYPE I19.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH, TO BE PLACED IN LAYERS NOT LESS THAN $2\frac{1}{2}$ " IN DEPTH OR GREATER THAN 4" IN DEPTH.
E1	PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5½" IN DEPTH.
Т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
W	WEDGING (SEE DETAIL SHOWING METHOD OF WEDGING).

NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.

862d03 RALEIGH, N.C. RAIL ON BRIDGE - SUB REGIONAL TIER STATE OF NORTH CAROLINA DEPT, OF TRANSPORTATION DIVISION OF HIGHWAYS GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO ENGLISH DETAIL DRAWING FOR STD. 6'-3" SPACING TRANSTION THE GUARDRAIL VERTICALLY FRO 1'-11" DOWN TO 1'-9" IN ONE 25' SECTION **T**0 III FOR AT SHOULDER BREAK

x 4" LIP CURB

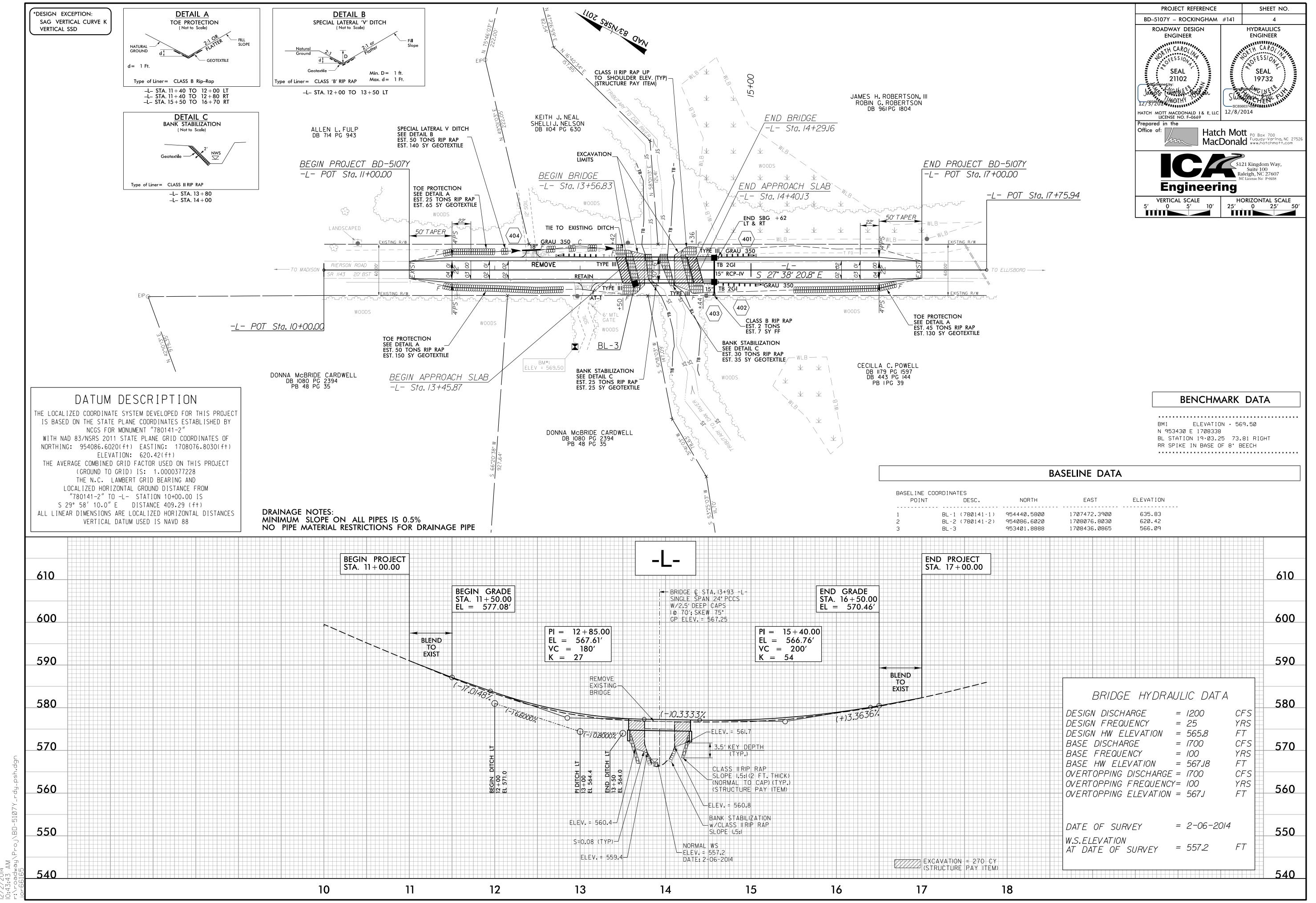
STRUCTURE PLANS TYPE SUB Ω C 4 **Σ** GUARDRAIL ANCHOR RAIL ON E SHOE PLAN STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. ENGLISH DETAIL DRAWING FOR 862d03 STRUCTURE ANCHOR UNITS GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO RAIL ON BRIDGE - SUB REGIONAL TIER



CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON	DATE: 06-22-12
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC.:	



12/2/2014

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" – HIGHWAY DESIGN BRANCH– N.C. DEPARTMENT OF TRANSPORTATION – RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD.	TITLE
1101.03	TEMPORARY ROAD CLOSURES
1110.01	STATIONARY WORK ZONE SIGNS
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS – LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS – TWO-LANE AND MULTI-LANE ROADWAYS
1205.12	PAVEMENT MARKINGS – BRIDGES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS – TYPES AND MOUNTING
1262.01	GUARDRAIL END DELINEATION

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

- B) PROVIDE PERMANENT SIGNING.
- C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

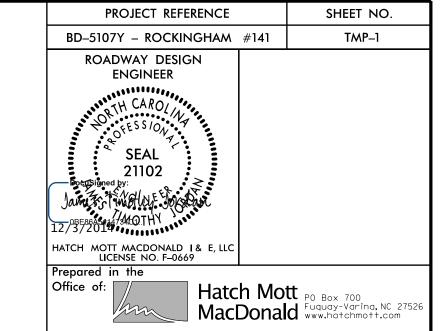
E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE.



PHASING

STEP 1: USING ROADWAY STANDARD DRAWING NUMBER 1101.03, SHEET 1

OF 9, AND SHEET TMP-2, PERFORM THE FOLLOWING:

- INSTALL ALL ROAD CLOSURE AND DETOUR SIGNING

INCLUDING BARRICADES

- CLOSE SR 1143 (RIERSON ROAD)

- PLACE TRAFFIC ONTO OFF- SITE DETOUR

TEXEL HOUTTE STATE SITE BETOOK

STEP 2: REMOVE EXISTING BRIDGE #141 AND CONSTRUCT THE PROPOSED BRIDGE AND APPROACHES AS SHOWN IN THE CONSTRUCTION

PLANS.

STEP 3: INSTALL FINAL PAVEMENT MARKINGS.

STEP 4: REMOVE ALL TRAFFIC CONTROL SIGNING AND DEVICES AND RE-OPEN SR 1143 (RIERSON ROAD) TO THE FINAL

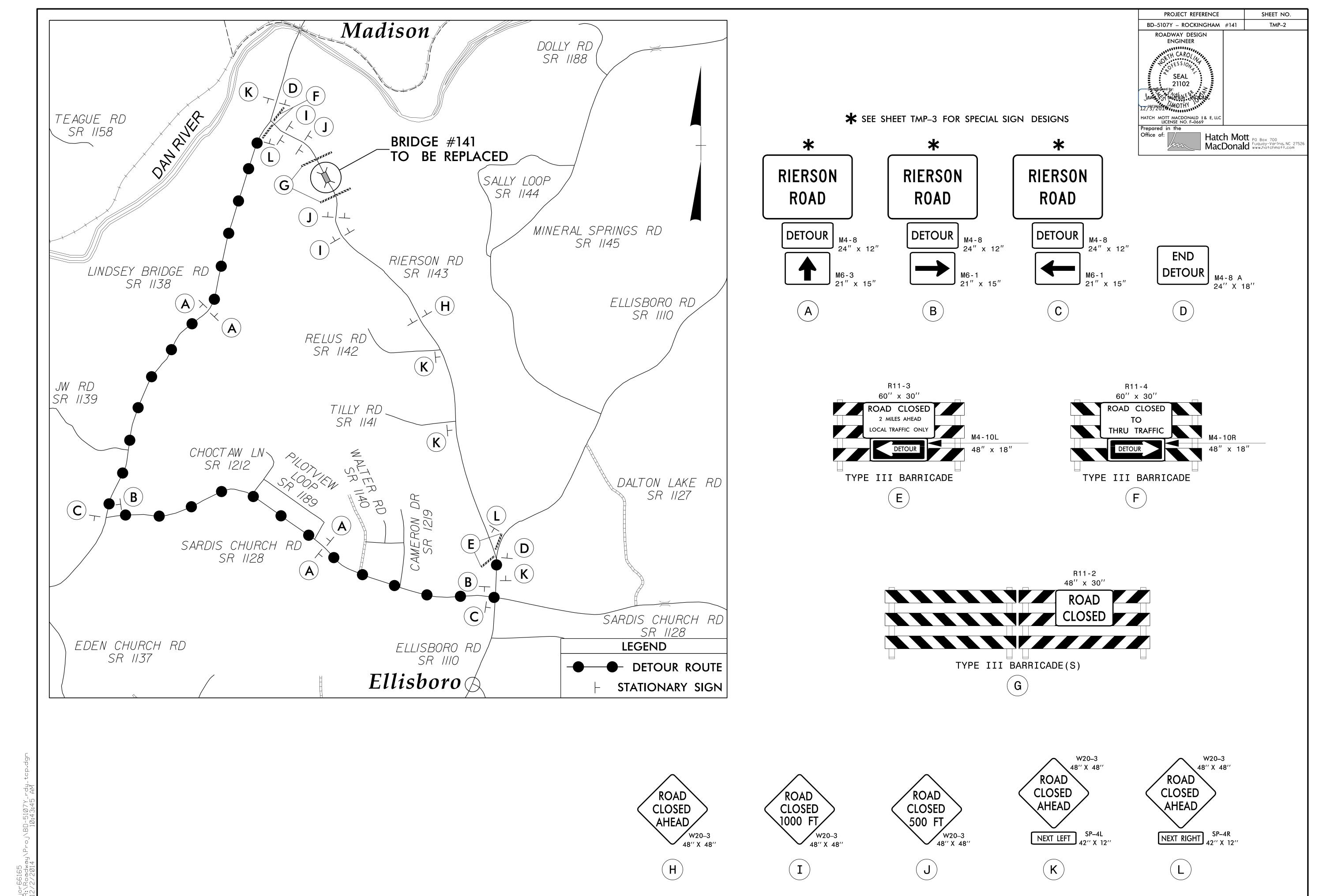
TRAFFIC PATTERN.

PAVEMENT MARKING

PAINT WHITE EDGELINE (4") 2400 LF PAINT YELLOW DOUBLE CENTER (4") 2400 LF

NOTE: QUANTITY INCLUDES 2 APPLICATIONS OF EACH

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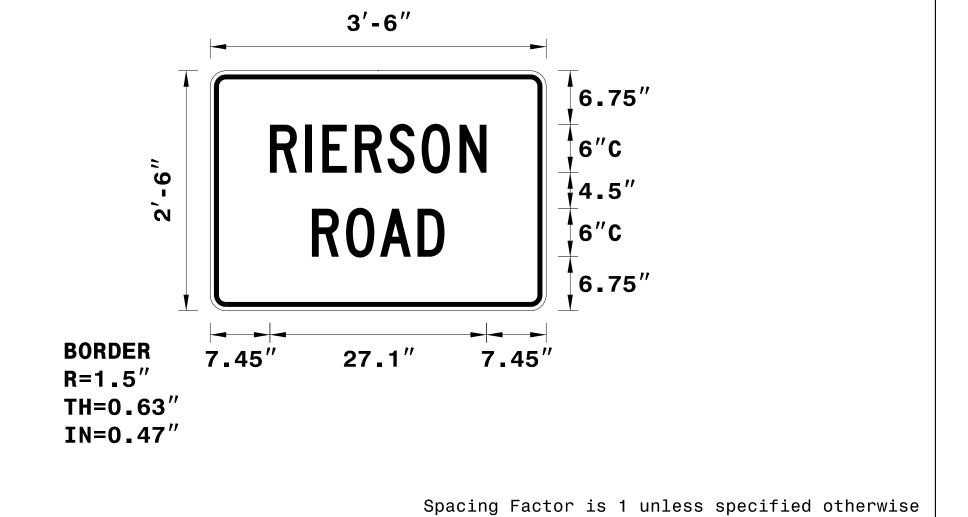


SIGN NUMBER: SD-1 BACKG COLOR: Fluorescent Orange TYPE: D COPY COLOR: Black QUANTITY: SEE PLANS SYMBOL X Y WID HT SIGN WIDTH: 3'-6" **HEIGHT: 2'-6"** TOTAL AREA: 8.8 Sq.Ft. **BORDER TYPE: INSET RECESS:** 0.47" WIDTH: 0.63" **RADII:** 1.5" MAT'L: 0.080" (2.0 mm) ALUMINUM NO. Z BARS:

USE NOTES: 1,2

- Legend and border shall be direct applied black non-reflective sheeting.
- 2.Background shall be NC GRADE B fluoresent orange retroreflective sheeting.

DESIGN BY: JTD CHECKED BY: NKP
DATE: Oct 28, 2013
PROJECT ID: BD-5107Y
DIV: 7



LETTER POSITIONS

LENGTH:

						PTT	Pr	เกตลา	INNS	ar	e nanei	edde 1	'n inwer	IPTT	cornei	~				Series
									10110	<u> </u>	c paner	cage	o lower							Text L
Ι	E	R	S	0	N															C 2
11.9	14	18.1	22.2	2 26.4	31.2															27
0	Α	D																		C 2
17	21.2	25.9																		16
																_				
	11.9	11.9 14 0 A	11.9 14 18.1 0 A D	11.9 14 18.1 22. 0 A D	11.9 14 18.1 22.2 26.4 0 A D	11.9 14 18.1 22.2 26.4 31.2 0 A D	11.9 14 18.1 22.2 26.4 31.2 O A D	11.9 14 18.1 22.2 26.4 31.2 O A D	11.9 14 18.1 22.2 26.4 31.2 O A D	11.9 14 18.1 22.2 26.4 31.2 0 A D	11.9 14 18.1 22.2 26.4 31.2 0 A D	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2	11.9 14 18.1 22.2 26.4 31.2

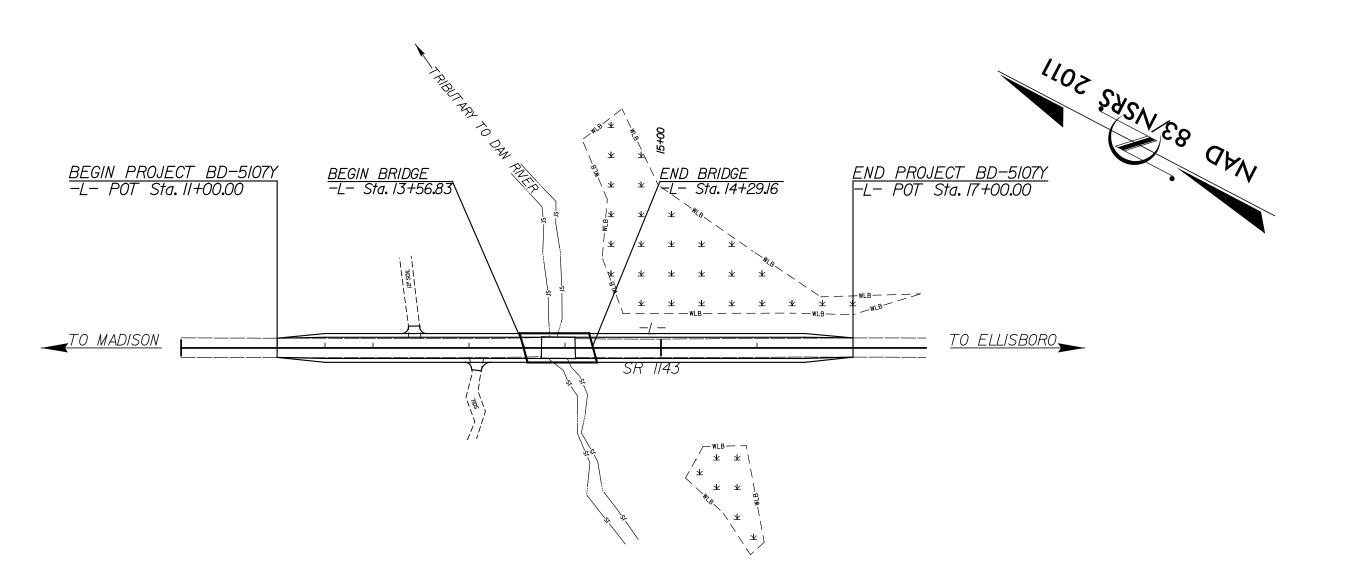
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

ROCKINGHAM COUNTY

BRIDGE NO. 141 ON SR 1143 OVER TRIBUTARY TO DAN RIVER



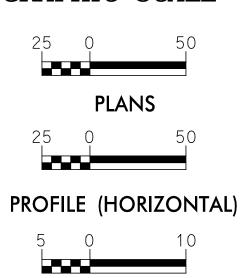
STATE	STA	STATE PROJECT REFERENCE NO.									
N.C.		BD=5107Y	EC=1								
STAT	E PROJ. NO.	F. A. PROJ. NO.	OJ. NO. DESCRIPTION								
4535	3.1.26	BRZ-1143(10)	P.E.								

EROSION	N AND SEDIMENT CONTROL MEASURES
<u>Std. #</u>	Description Symbol
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion TD
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains
1630.02	Silt Basin Type B
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type-B
	Wattle / Coir Fiber Wattle
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)
1634.01	Temporary Rock Sediment Dam Type-A
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A
1635.02	Rock Pipe Inlet Sediment Trap Type-B
1630.04	Stilling Basin
1630.06	Special Stilling Basin
	Rock Inlet Sediment Trap:
1632.01	Туре А
1632.02	Туре В
1632.03	Туре С
	Skimmer Basin
	Tiered Skimmer Basin
	Infiltration Basin

THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION.



GRAPHIC SCALE



PROFILE (VERTICAL)

ROADSIDE ENVIRONMENTAL UNIT **DIVISION OF HIGHWAYS** STATE OF NORTH CAROLINA

LEVEL III CERTIFIED BY: ALEXANDER SNIDER, P.E. CERTIFICATION NUMBER: 3064 ISSUED: AUGUST 4, 2014

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

Prepared in the Office of:

ICA ENGINEERING

5121 KINGDOM WAY, SUITE 100 RALEIGH NC 27607 NC License No: F-0258

2012 STANDARD SPECIFICATIONS

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2012 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains 1630.01 Riser Basin

1630.02 Silt Basin Type B 1630.03 Temporary Silt Ditch

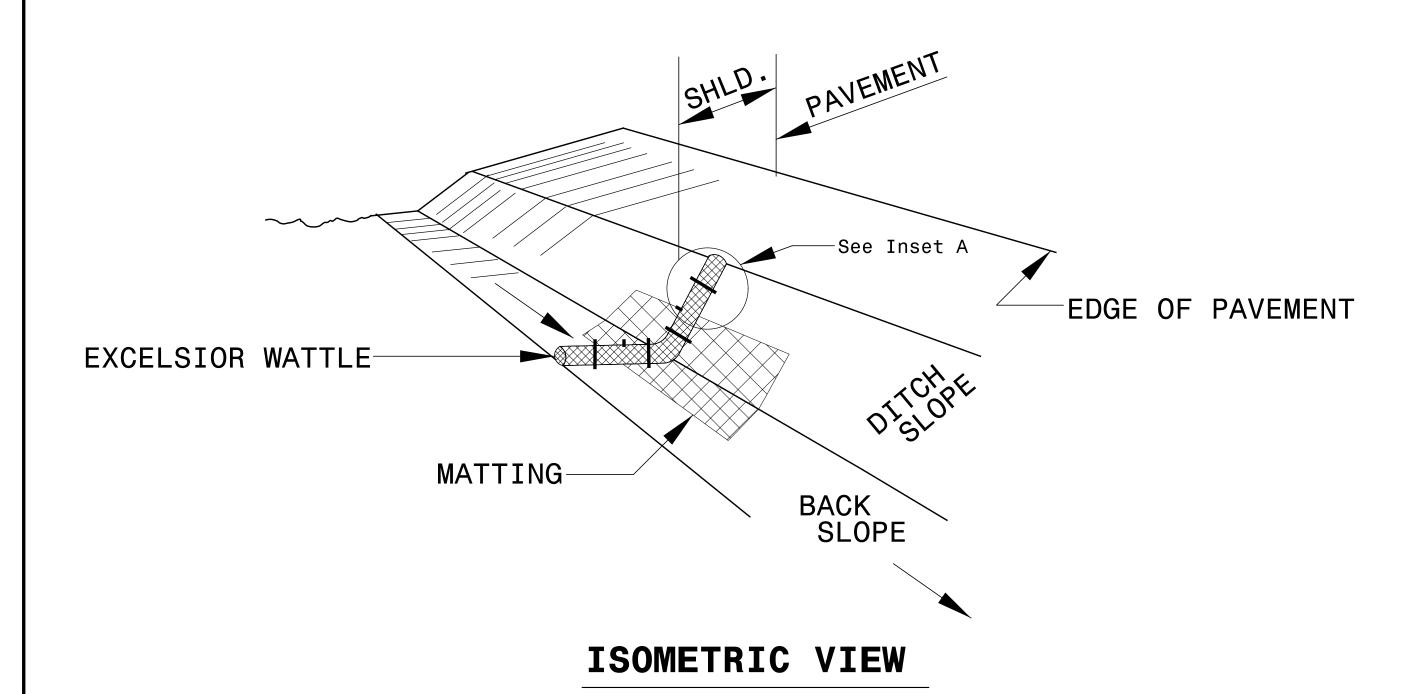
1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation

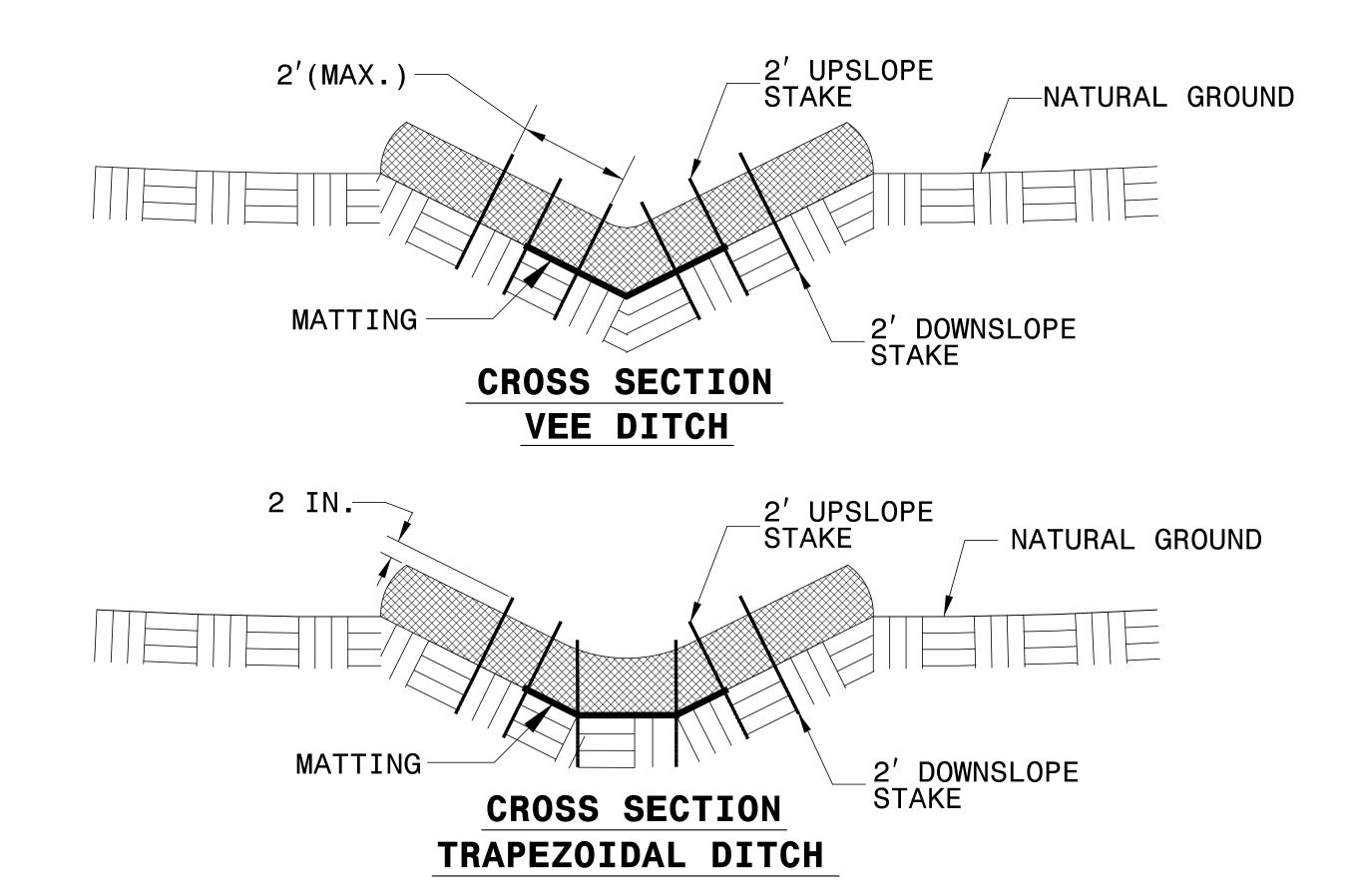
1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A 1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

1645.01 Temporary Stream Crossing

PROJECT REFERENCE NO.SHEET NO.BD-5/07YEC-2

WATTLE DETAIL





NOTES:

USE MINIMUM 12 IN. DIAMETER EXCELSIOR WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

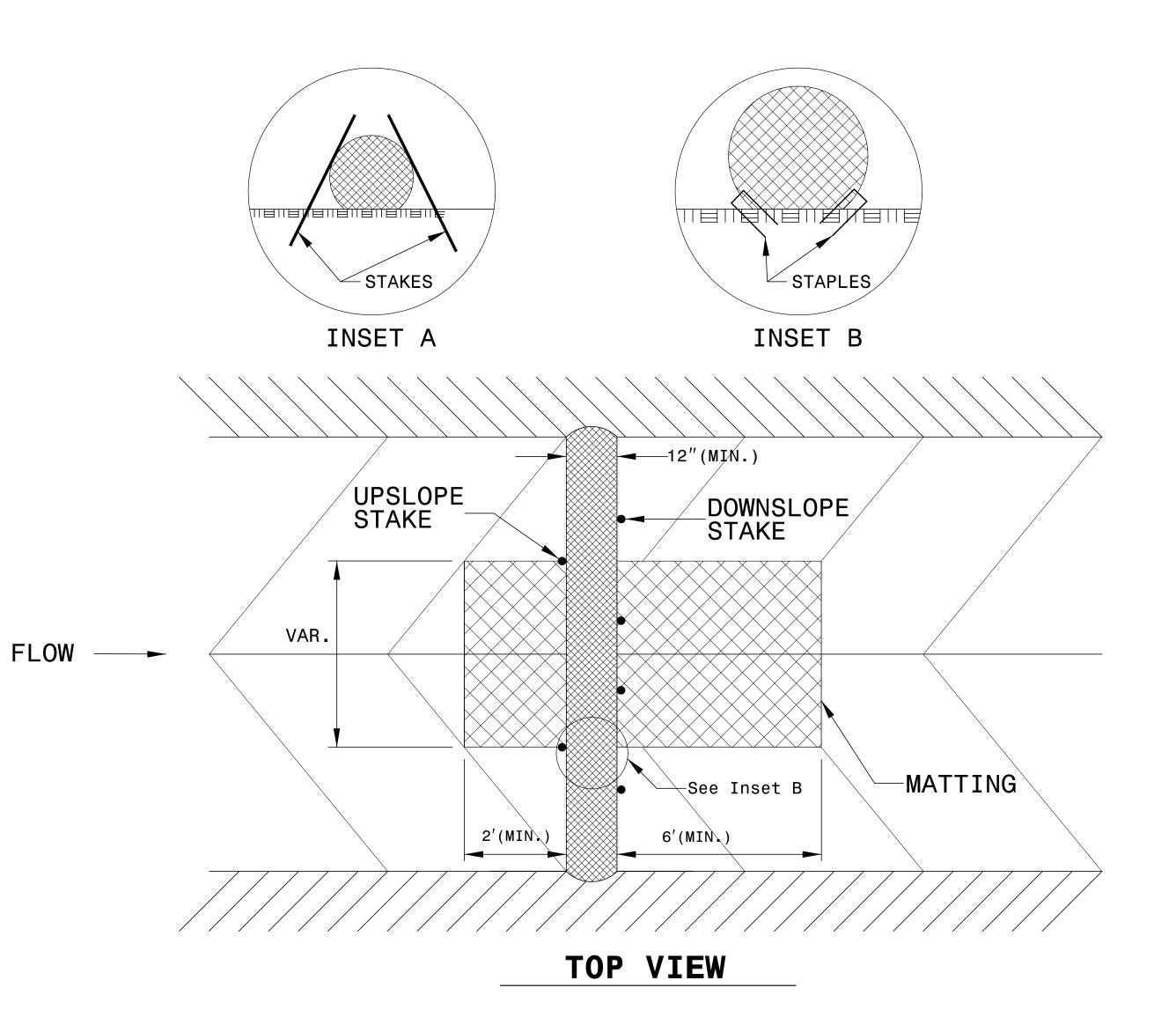
ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

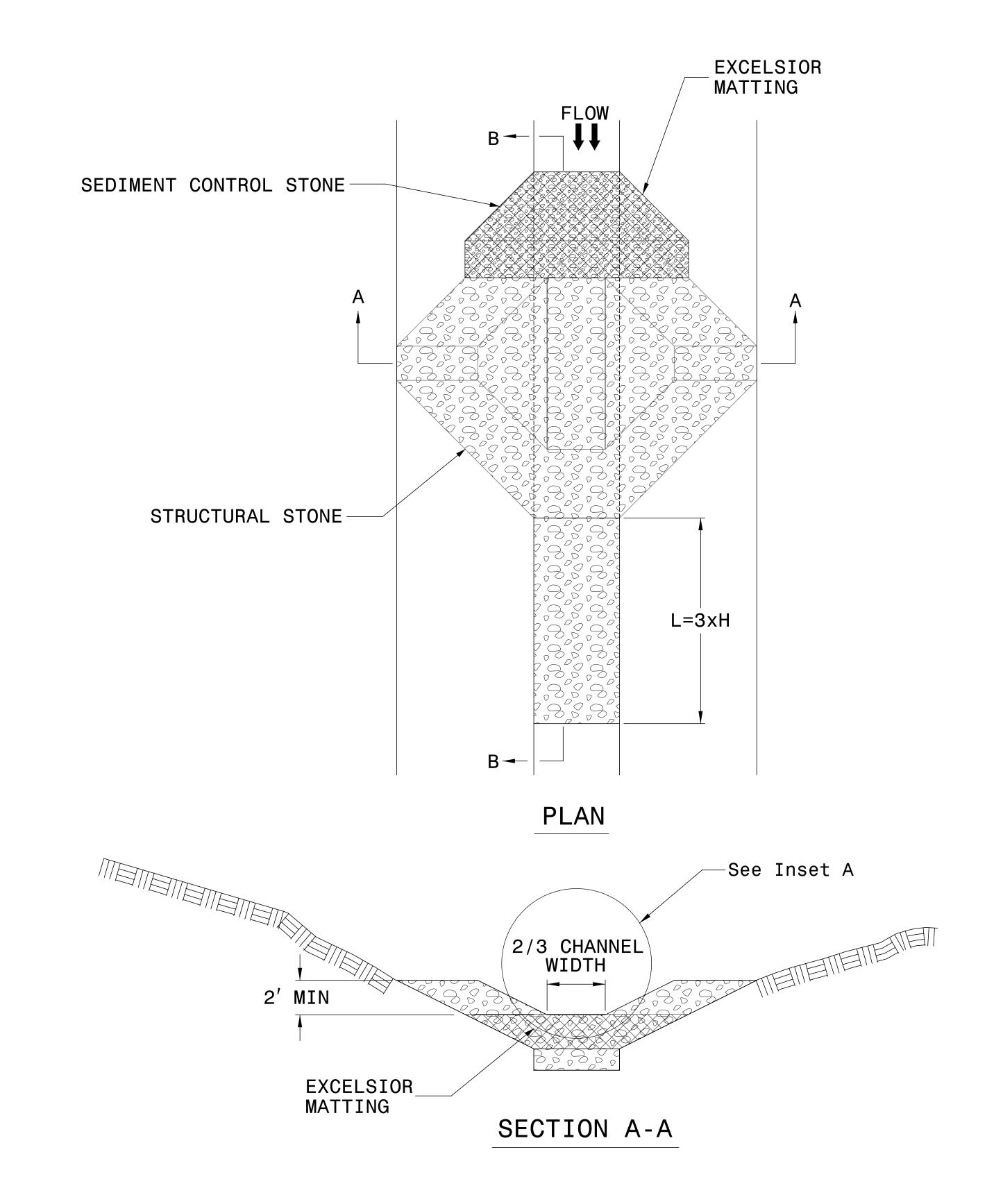
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



PROJECT REFERENCE NO. SHEET NO. BD-5/07Y EC-2A

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

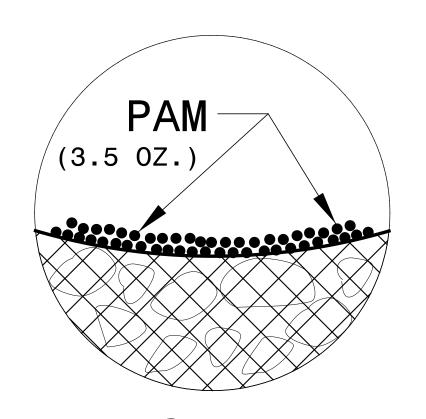


NOTES

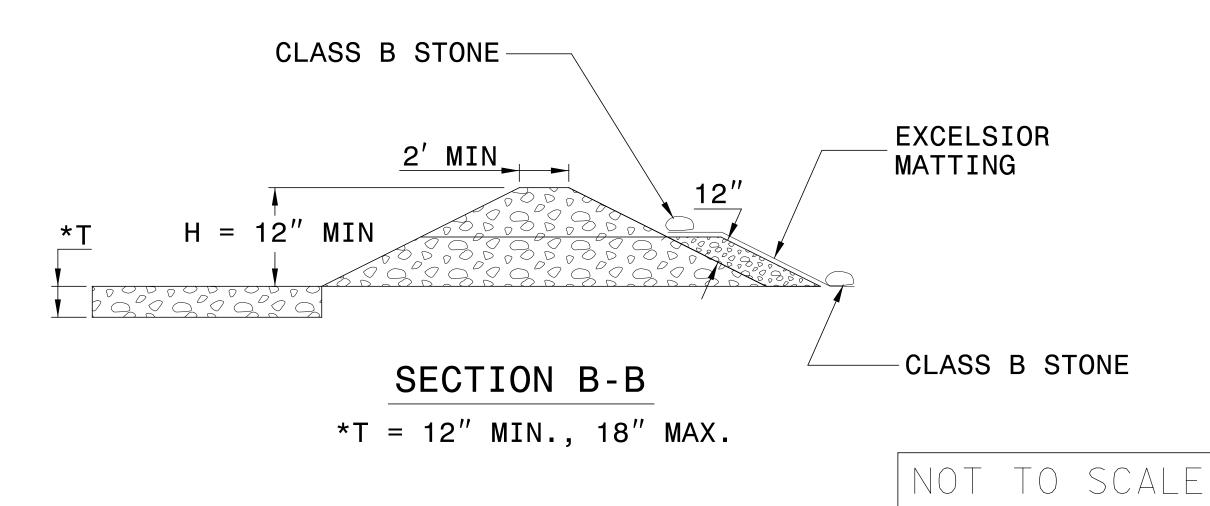
USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 3.5 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



INSET A



PROJECT REFERENCE NO.SHEET NO.BD-5/07YEC-3

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

GRAPHIC SCALE

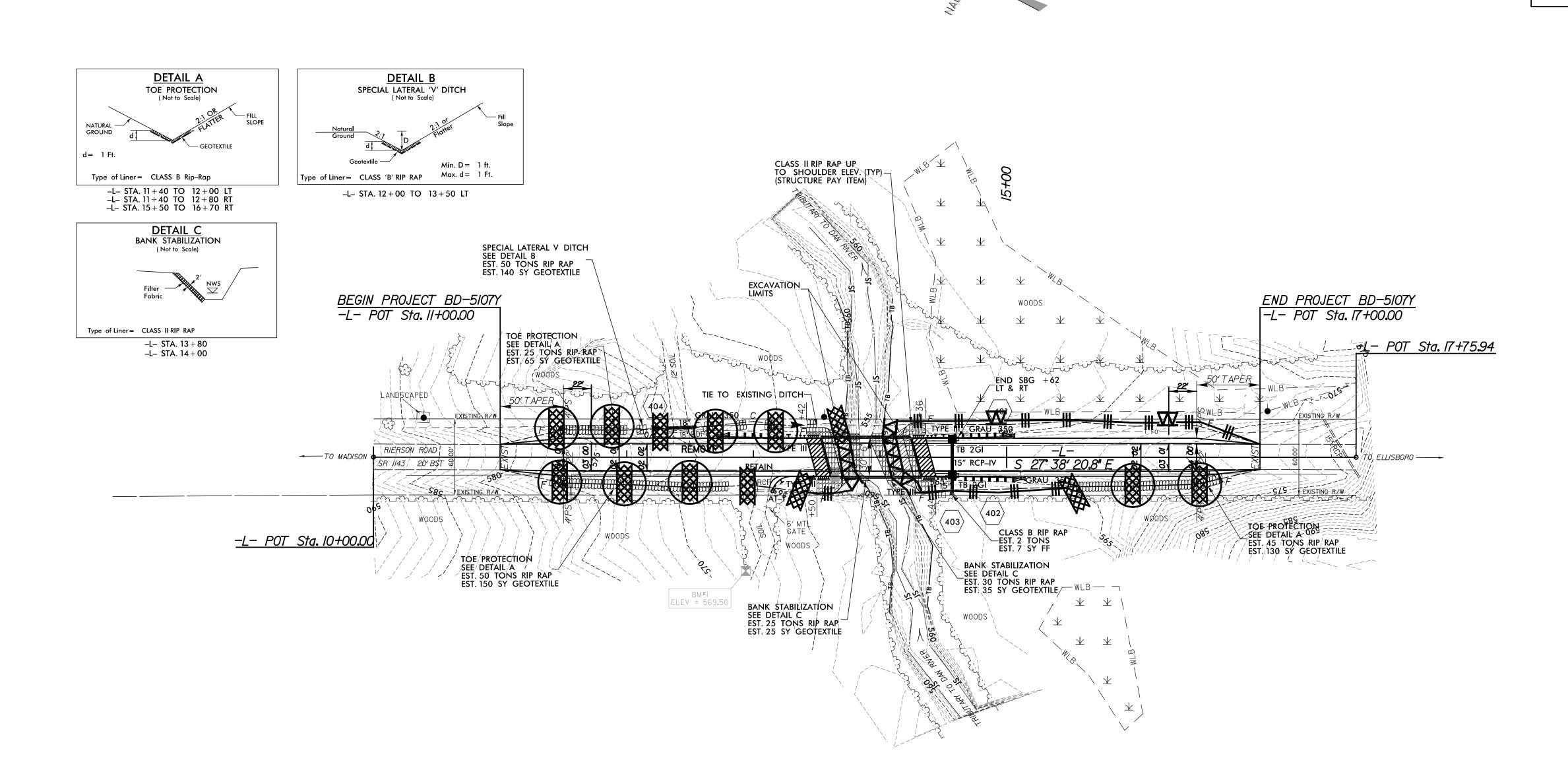
50 0 50 100

PROJECT REFERENCE NO. SHEET NO. $BD-5/07Y \qquad EC-\underline{4/CONST.4}$ RW SHEET NO.

ROADSIDE ENVIRONMENTAL PROJECT ENGINEER

LEVEL III CERTIFIED BY:
ALEXANDER SNIDER, P.E.
CERTIFICATION NUMBER: 3064
ISSUED: AUGUST 4, 2014

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 4



DRAINAGE NOTES: MINIMUM SLOPE ON ALL PIPES IS 0.5% NO PIPE MATERIAL RESTRICTIONS FOR DRAINAGE PIPE

IOTE:

ALL EROSION CONTROL DEVICES SHOWN ARE LOCATED WITHIN EXISTING/PROPOSED R/W OR EASEMENT.



GRAPHIC SCALE

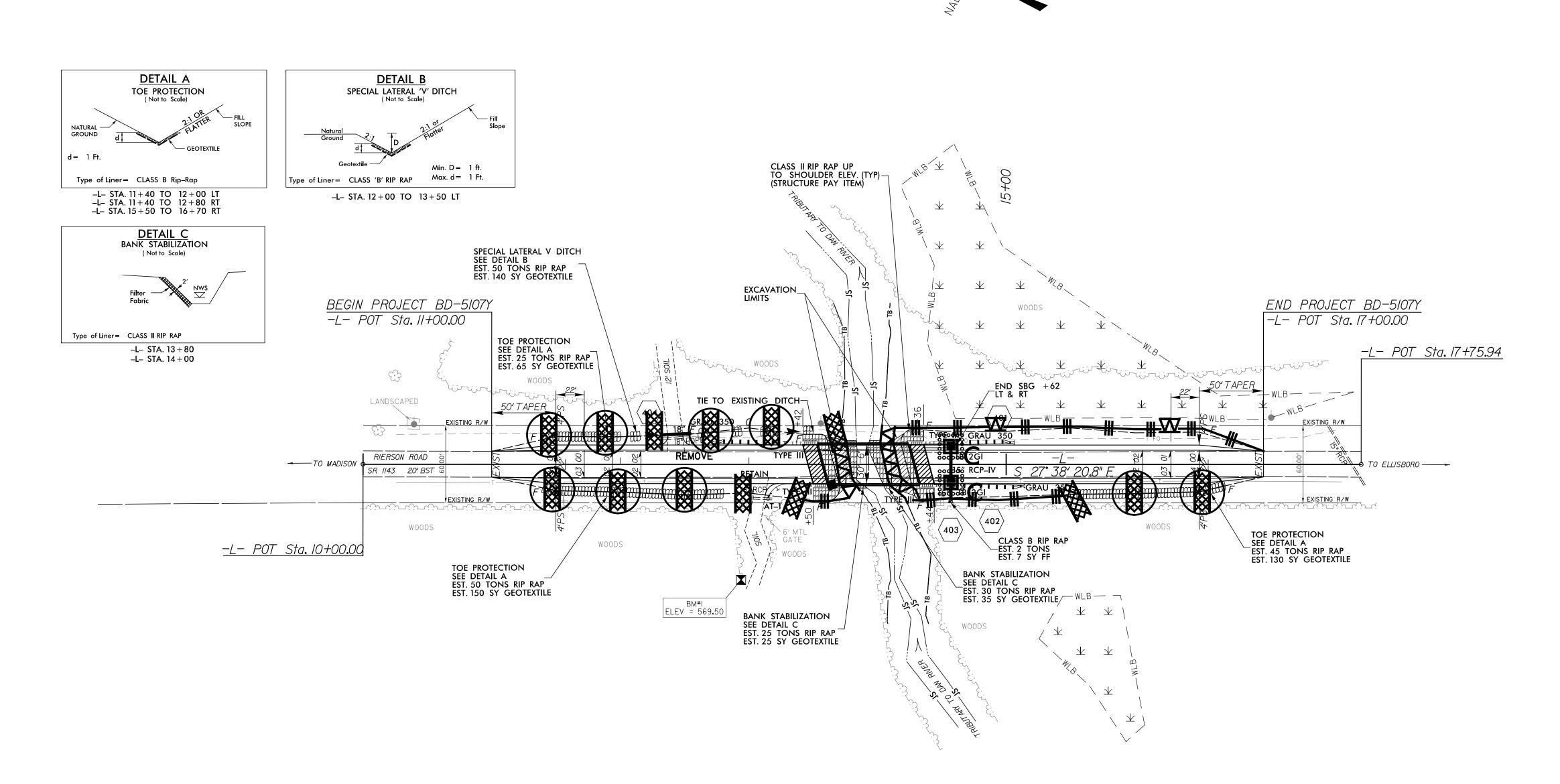
50 0 50 100

PROJECT REFERENCE NO. SHEET NO. $BD-5/07Y \qquad EC-5/CONST.4$ RW SHEET NO.

ROADSIDE ENVIRONMENTAL PROJECT ENGINEER

LEVEL III CERTIFIED BY:
ALEXANDER SNIDER, P.E.
CERTIFICATION NUMBER: 3064
ISSUED: AUGUST 4, 2014

FINAL EROSION CONTROL FOR CONSTRUCTION SHEET 4



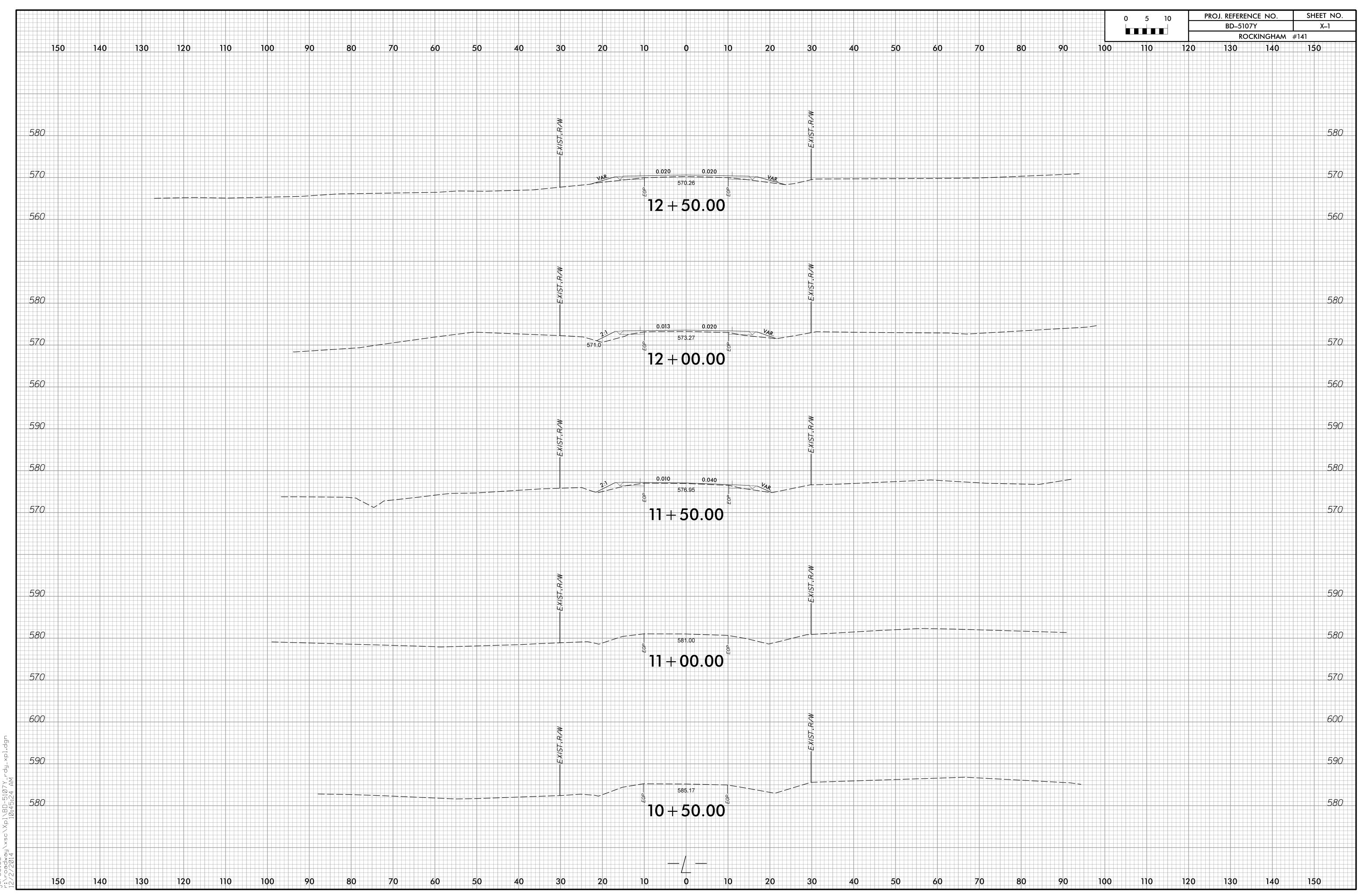
DRAINAGE NOTES: MINIMUM SLOPE ON ALL PIPES IS 0.5% NO PIPE MATERIAL RESTRICTIONS FOR DRAINAGE PIPE

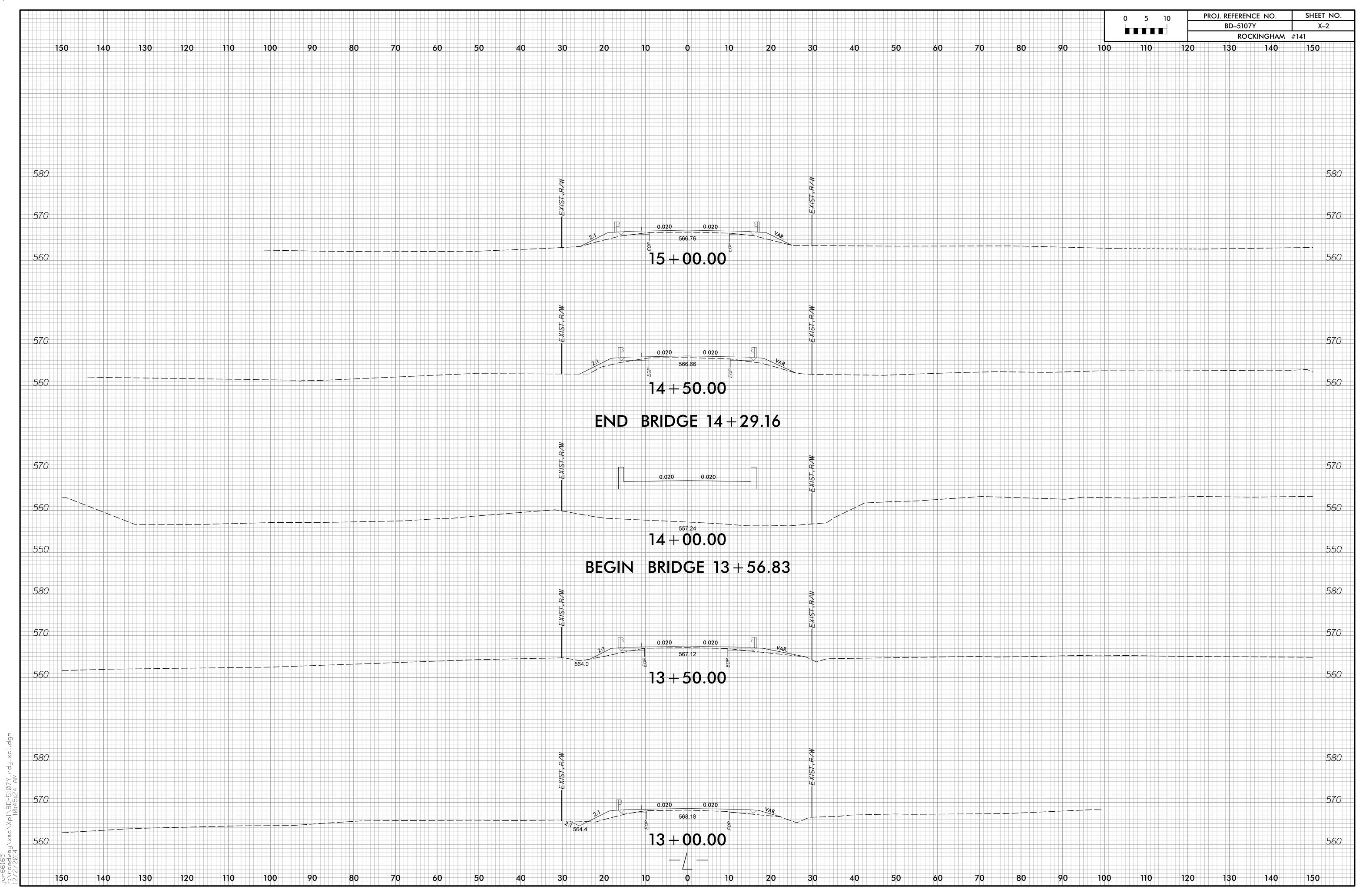
NOTE:

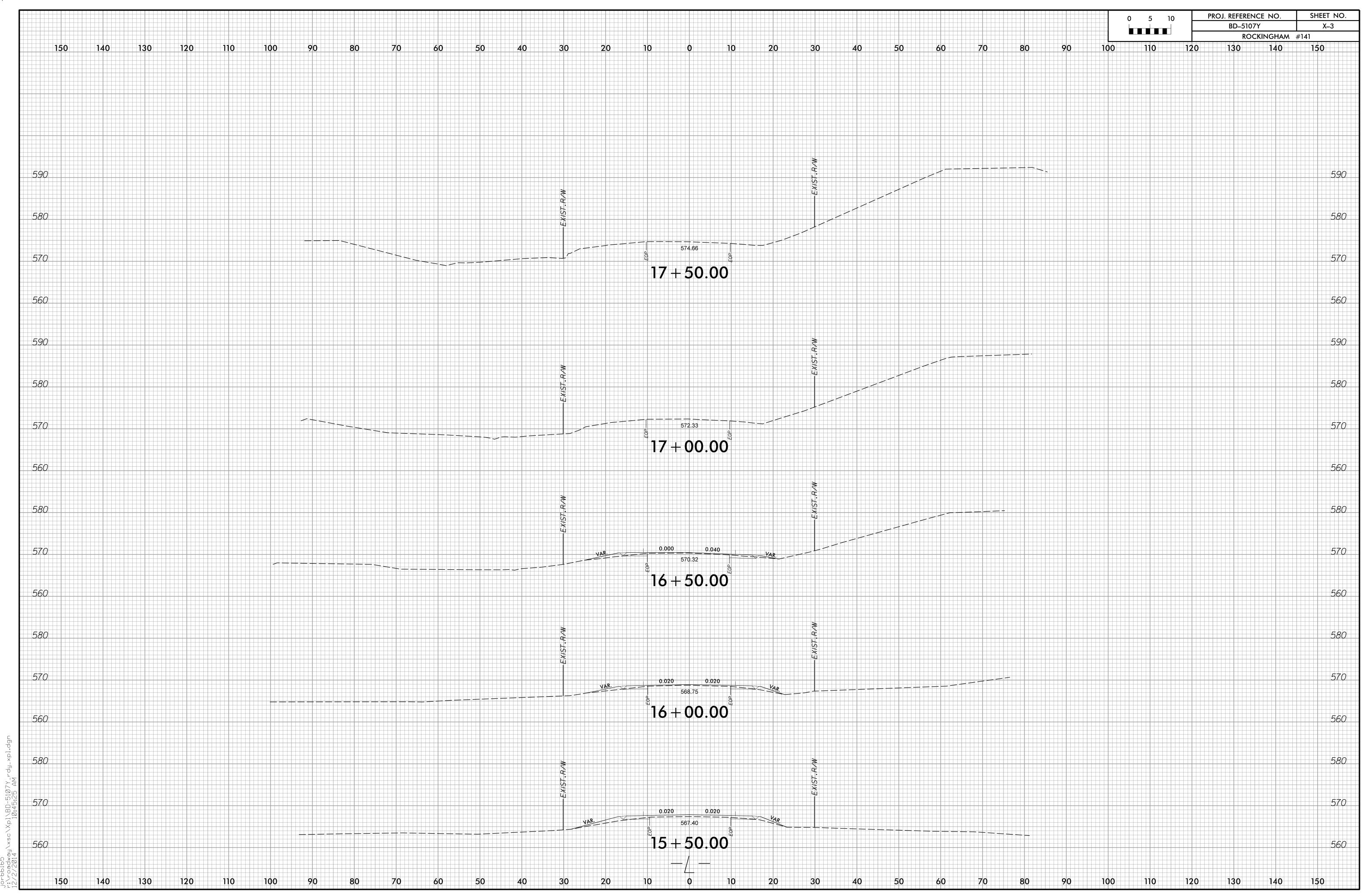
ALL EROSION CONTROL DEVICES SHOWN ARE LOCATED WITHIN EXISTING/PROPOSED R/W OR EASEMENT.

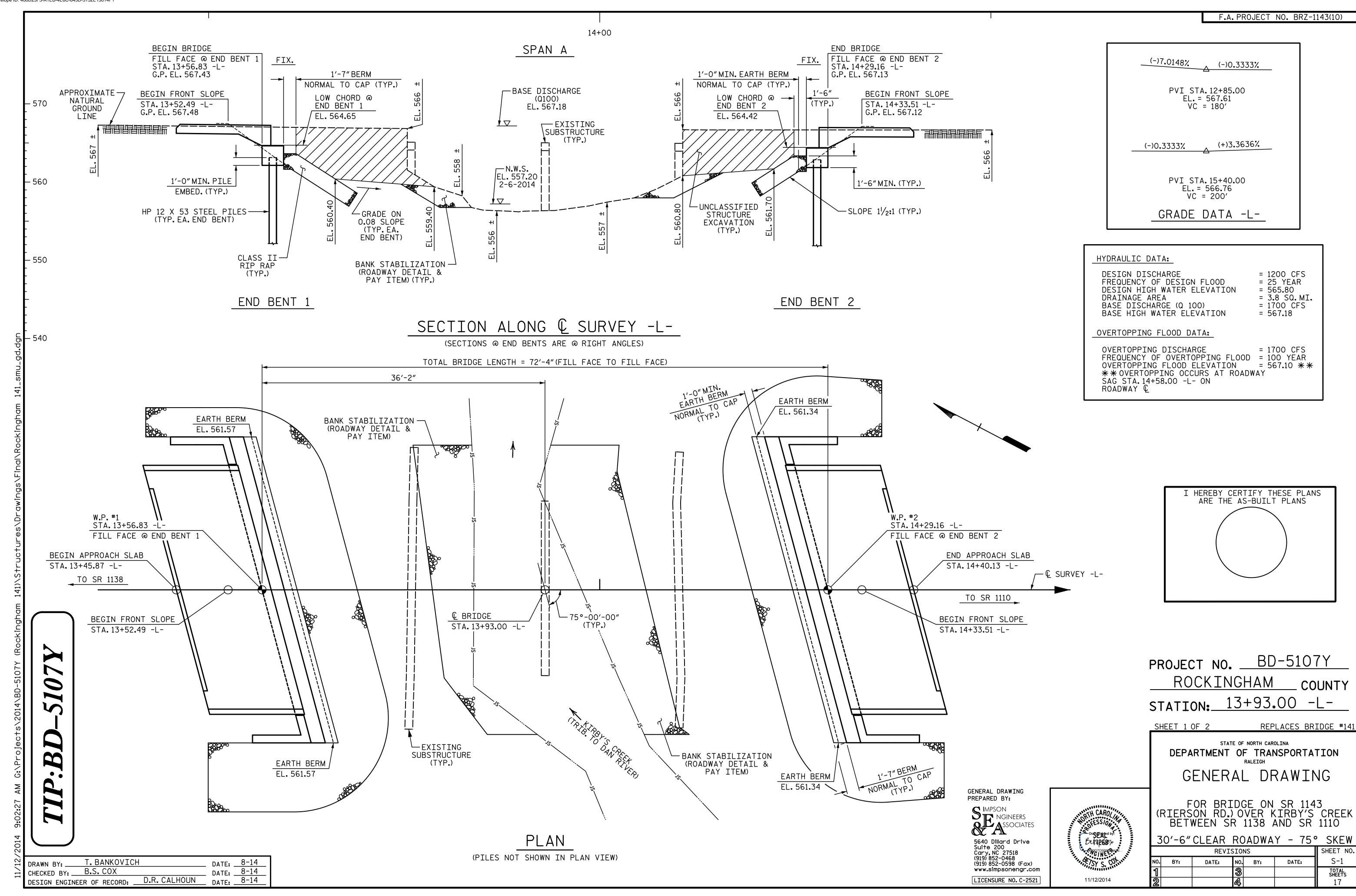


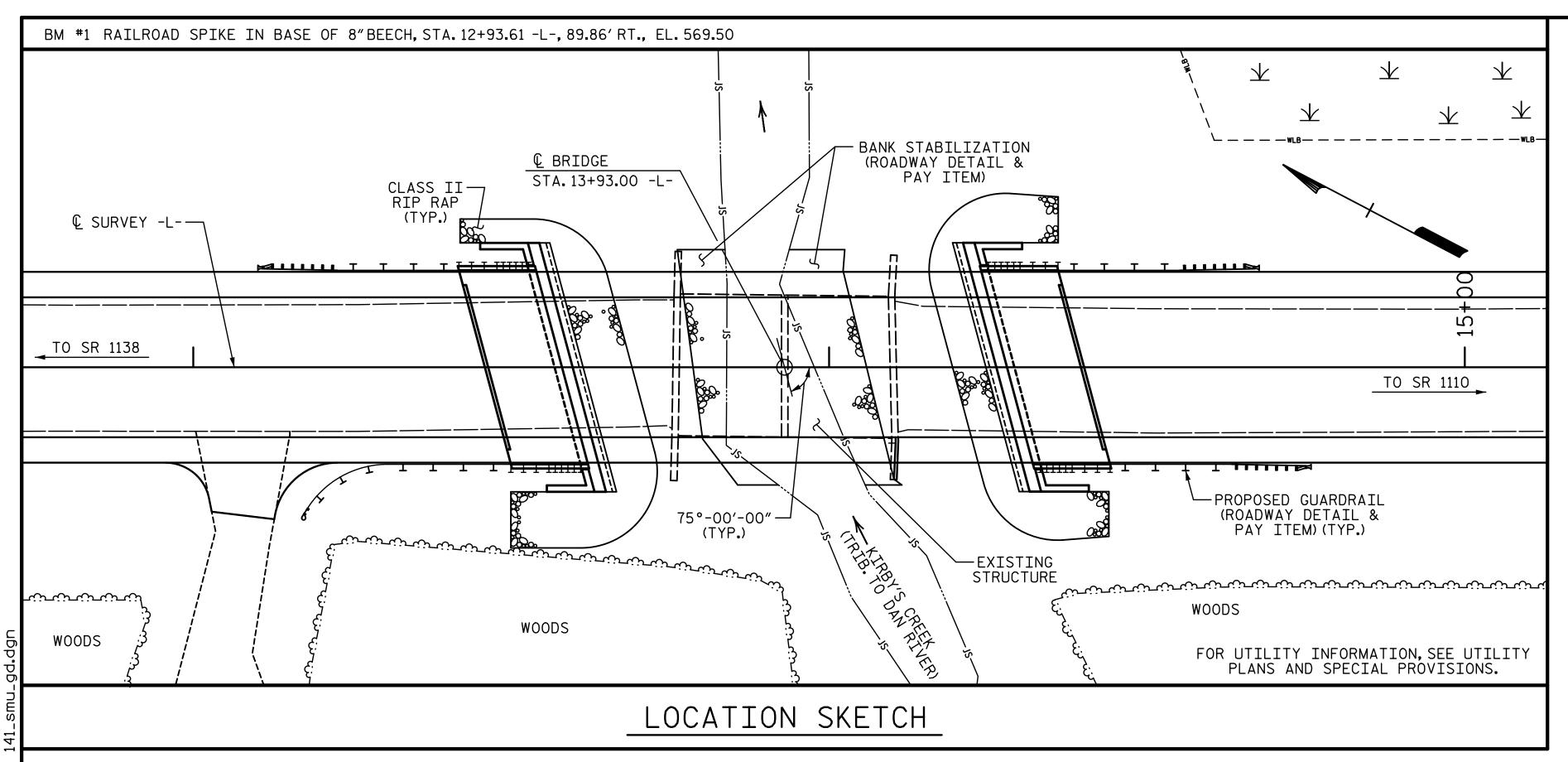
:/Lydraulics/Erosion Control\cadd\BD-5107Y_hyd_er o & Associates,P.C.











	TOTAL BILL OF MATERIAL ————————————————————————————————————														
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	NG HP 12 X 53 STEEL PILES		TWO BAR METAL RAIL	1'-2" X 3'-0" CONCRETE PARAPET	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASOMERIC BEARINGS	PREST CONCRET	(2'-0" RESSED E CORED ABS	
	LS	LS	CY	LS	LB	NO.	LF	LF	LF	TON	SY	LS	NO.	LF	
SUPERSTRUCTURE				LS				124.38	140.00			LS	11	770.00	
END BENT 1		LS	14.8		2193	7	140			105	116				
END BENT 2		LS	14.8		2193	7	160			76	85				
TOTAL	LS	LS	29.6	LS	4386	14	300	124.38	140.00	181	201	LS	11	770.00	

PREPARED BY: RISTORIAL SERVICE ASSOCIATES 5640 Dillard Drive Suite 200 Cary, NC 27518 (919) 852-0468 (919) 852-0598 (Fax) www.simpsonengr.com

GENERAL DRAWING

LICENSURE NO. C-2521 11/12/2014 PROJECT NO. BD-5107Y ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

FOR BRIDGE ON SR 1143 (RIERSON RD.) OVER KIRBY'S CREEK BETWEEN SR 1138 AND SR 1110

30'-6"CLEAR ROADWAY - 75° SKEW

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			17

T. BANKOVICH CHECKED BY: B.S. COX DATE: _ DATE: 8-14 DESIGN ENGINEER OF RECORD: D.R. CALHOUN

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS,

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25 FEET LEFT AND 30 FEET RIGHT OF CENTERLINE OF ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTS OF 1 SPAN @ 17'-7" AND 1 SPAN @ 17'-8" WITH A CLEAR ROADWAY WIDTH OF 21.167 FEET. THE SUPERSTRUCTURE CONSISTS OF A TIMBER DECK ON TIMBER JOISTS. THE ABUTMENTS ARE TIMBER CAPS AND PILES WITH TIMBER BULKHEADS AND THE INTERIOR BENT IS A TIMBER CAP ON TIMBER PILES. THE EXISTING STRUCTURE IS LOCATED AT THE SITE OF THE PROPOSED STRUCTURE AND SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE. DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIN	MIT S	ГАТЕ				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT]
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.014		1.75	0.269	1.04	70′	EL	34.482	0.608	1.10	70′	EL	3.448	0.80	0.269	1.01	70′	EL	34.482	
DESIGN		HL-93(0pr)	N/A		1.355		1.35	0.269	1.35	70′	EL	34.482	0.608	1.43	70′	EL	3.448	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.315	47.356	1.75	0.269	1.36	70′	EL	34.482	0.608	1.38	70′	EL	3.448	0.80	0.269	1.32	70′	EL	34.482	
MATINO		HS-20(0pr)	36.000		1.757	63 . 236	1.35	0.269	1.76	70'	EL	34.482	0.608	1.79	70′	EL	3.448	N/A						
		SNSH	13.500		2.938	39.656	1.40	0.269	3.78	70'	EL	34.482	0.608	4.12	70′	EL	3.448	0.80	0.269	2.94	70′	EL	34.482	
		SNGARBS2	20.000		2.203	44.052	1.40	0.269	2.84	70′	EL	34.482	0.608	2.93	70′	EL	3.448	0.80	0.269	2.20	70′	EL	34.482	
		SNAGRIS2	22.000		2.092	46.016	1.40	0.269	2.69	70′	EL	34.482	0.608	2.72	70'	EL	3.448	0.80	0.269	2.09	70′	EL	34.482	
		SNCOTTS3	27 . 250		1.462	39.844	1.40	0.269	1.88	70′	EL	34.482	0.608	2.06	70'	EL	3.448	0.80	0.269	1.46	70′	EL	34.482	
	NS	SNAGGRS4	34.925		1.227	42 . 856	1.40	0.269	1.58	70′	EL	34.482	0.608	1.71	70′	EL	3.448	0.80	0.269	1.23	70′	EL	34.482	
		SNS5A	35 . 550		1.200	42.646	1.40	0.269	1.54	70′	EL	34.482	0.608	1.73	70′	EL	3.448	0.80	0.269	1.20	70′	EL	34.482	
		SNS6A	39 . 950		1.103	44.058	1.40	0.269	1.42	70′	EL	34.482	0.608	1.58	70′	EL	3.448	0.80	0.269	1.10	70′	EL	34.482	
LEGAL		SNS7B	42.000		1.050	44.113	1.40	0.269	1.35	70′	EL	34.482	0.608	1 . 55	70′	EL	3.448	0.80	0.269	1.05	70′	EL	34.482	
LOAD RATING		TNAGRIT3	33.000		1.345	44.401	1.40	0.269	1.73	70′	EL	34.482	0.608	1.88	70'	EL	3.448	0.80	0.269	1.35	70′	EL	34.482	
INATING		TNT4A	33.075		1.352	44.717	1.40	0.269	1.74	70′	EL	34.482	0.608	1.83	70′	EL	3.448	0.80	0.269	1.35	70′	EL	34.482	
		TNT6A	41.600		1.108	46.073	1.40	0.269	1.43	70′	EL	34.482	0.608	1.65	70′	EL	3.448	0.80	0.269	1.11	70′	EL	34.482	
	TS.	TNT7A	42.000		1.114	46.794	1.40	0.269	1.43	70′	EL	34.482	0.608	1.62	70′	EL	3.448	0.80	0.269	1.11	70′	EL	34.482	
		TNT7B	42.000		1.155	48 . 526	1.40	0.269	1.49	70′	EL	34.482	0.608	1 . 51	70′	EL	3.448	0.80	0.269	1.16	70′	EL	34.482	
		TNAGRIT4	43.000		1.097	47.174	1.40	0.269	1.41	70′	EL	34.482	0.608	1.46	70′	EL	3.448	0.80	0.269	1.10	70′	EL	34.482	
		TNAGT5A	45.000		1.033	46.505	1.40	0.269	1.33	70′	EL	34.482	0.608	1.45	70′	EL	3.448	0.80	0.269	1.03	70′	EL	34.482	
		TNAGT5B	45.000	3	1.020	45 . 905	1.40	0.269	1.31	70′	EL	34.482	0.608	1.39	70′	EL	3.448	0.80	0.269	1.02	70′	EL	34.482	

LOAD FACTORS:

	DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
	LOAD RATING FACTORS	STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

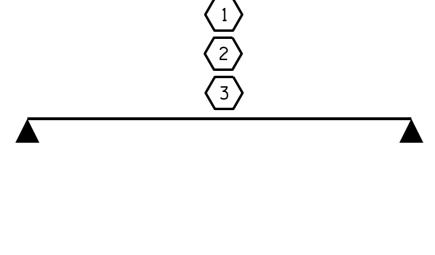
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- ** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER



<u>LRFR SUMMARY</u>

FOR SPAN 'A'

ASSEMBLED BY: A.C. OUTLAW DATE: 5/06/14 CHECKED BY: W.F. PARKER DATE: 5/08/14

DRAWN BY: CVC 6/IO CHECKED BY: DNS 6/IO PROJECT NO. BD-5107Y
ROCKINGHAM county
STATION: 13+93.00 -L-

STATE OF NORTH CAROLINA

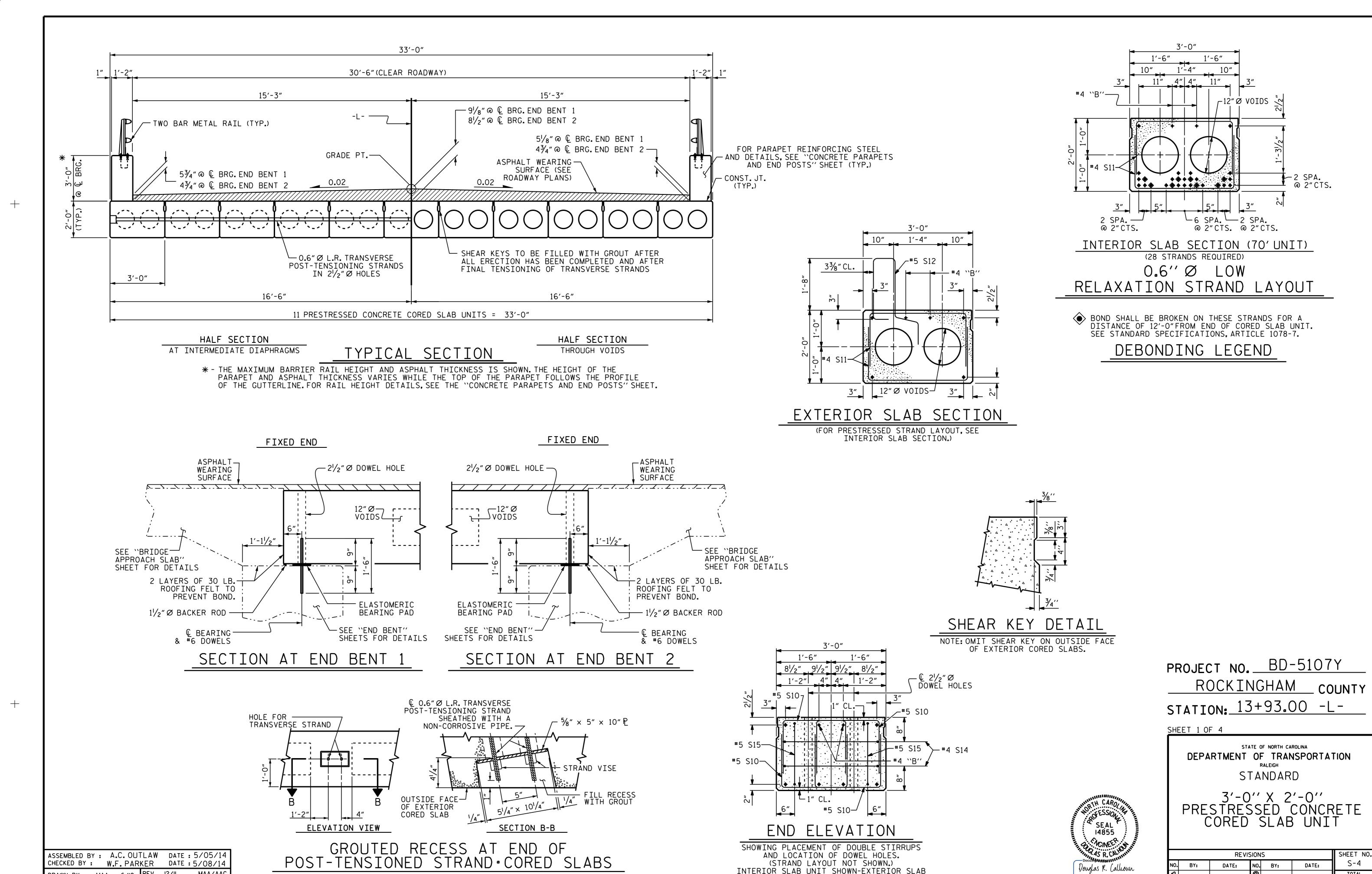
STANDARD

LRFR SUMMARY FOR
70' CORED SLAB UNIT
75° SKEW

(NON-INTERSTATE TRAFFIC)

	REVI	SION	1S		SHEE
BY:	DATE:	NO.	BY:	DATE:	S
		3			TO SHI





STD. NO. 24PCS4_33_75S

11/21/2014

DATE:

SHEET NO.

S-4

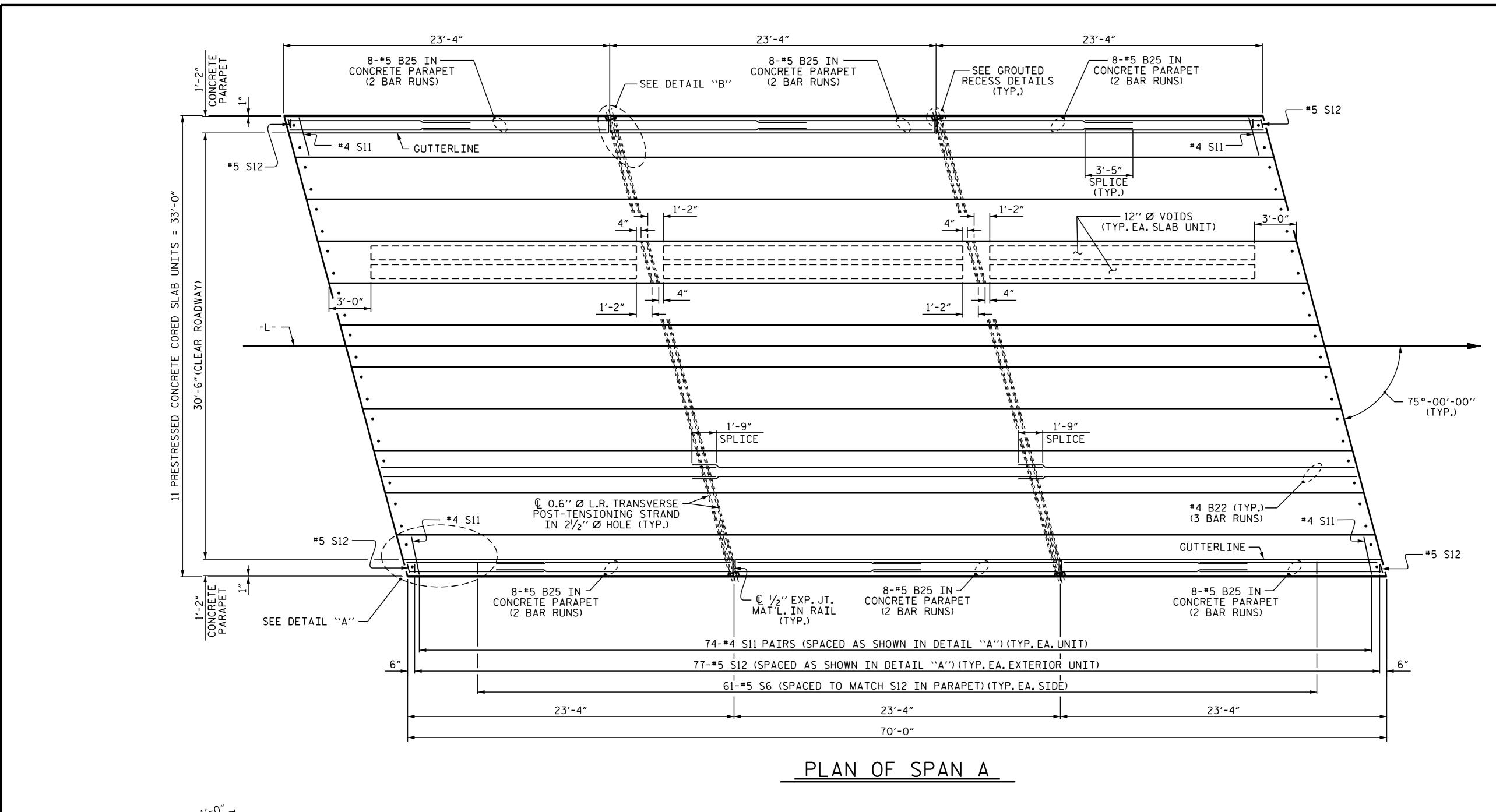
2 SPA. @ 2"CTS.

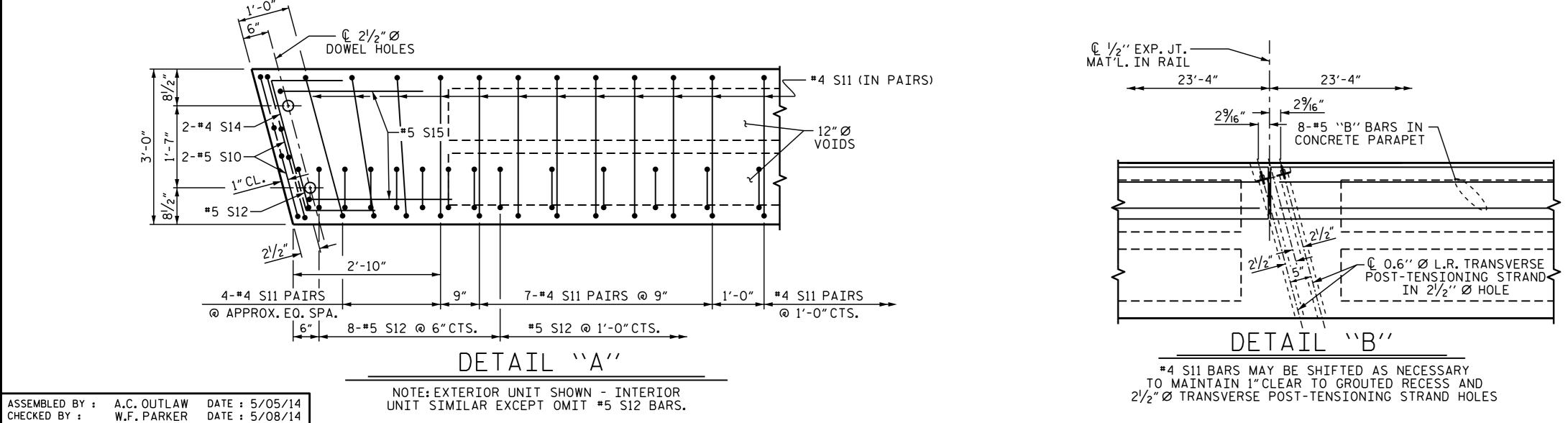
DRAWN BY: MAA 6/10 REV. 12/11

CHECKED BY : MKT 7/10

MAA/AAC

INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.





PROJECT NO. BD-5107Y

ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

SHEET 2 OF 4

SEAL 14855

O SOLINEER OF

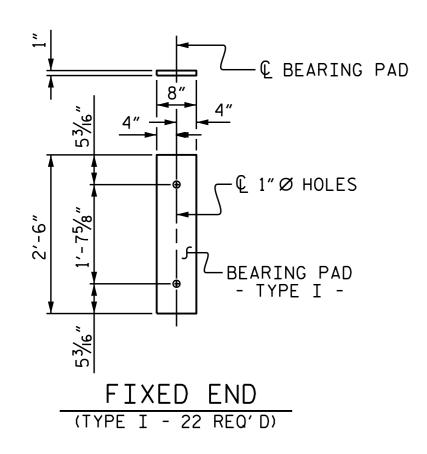
12EFE12F0292440... 11/21/2014 DEPARTMENT OF TRANSPORTATION
RALEIGH

PLAN OF 70'UNIT 30'-6"CLEAR ROADWAY 75° SKEW

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-5
0			3			TOTAL SHEETS
2			4 3			17

DRAWN BY: MAA 6/10 REV. 12/5/II MAA/AAC

CHECKED BY : MKT 7/10



ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

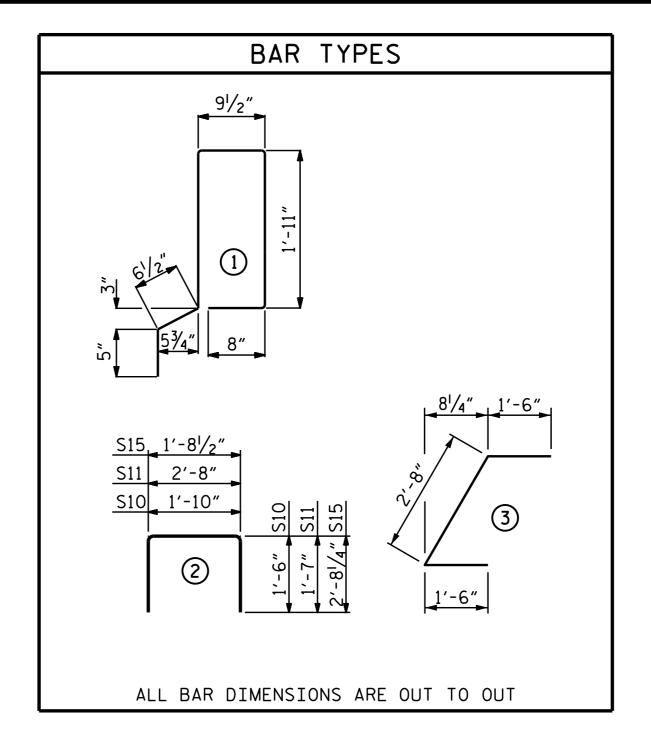
CONCRETE	RELEA	4SE	STRENGTH
UNIT			PSI
70'UNITS			5500

GRADE 270 STRANDS						
	0.6"Ø L.R.					
AREA (SQUARE INCHES)	0.217					
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600					
APPLIED PRESTRESS (LBS.PER STRAND)	43,950					

CORED SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH				
EXTERIOR C.S.	2	70'-0"	140'-0"				
INTERIOR C.S.	9	70′-0″	630'-0"				
TOTAL	11	70'-0"	770'-0"				

BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT							
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B22	6	#4	STR	24'-6"	98	24'-6"	98
S10	8	# 5	2	4'-10"	40	4'-10"	40
S11	148	#4	2	5′-10″	577	5′-10″	577
* S12	79	#5	1	6′-3″	515		
S14	4	#4	3	5′-8″	15	5′-8″	15
S15	4	#5	2	7'-1"	30	7'-1"	30
REINFO	ORCING S	STEEL	LBS	S.	760		760
	Y COATE						
	REINFORCING STEEL LBS. 515						
7000 F	P.S.I. CO	<u>NCRETE</u>	CU. YDS)•	12.0		12.0
0.6"Ø	L.R. STR.	ANDS	No).	28		28

GUTTERLINE ASPH	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
70' UNITS	1 1/4"	2'-81/2"



NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN CONCRETE PARAPETS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	4 ⁵ ⁄ ₁₆ ″ ∤
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	¹³ / ₁₆ " ♦
FINAL CAMBER	31/2″ ╽

** INCLUDES FUTURE WEARING SURFACE

PROJECT NO. BD-5107Y ROCKINGHAM COUNTY STATION: 13+93.00 -L-

SHEET 3 OF 4

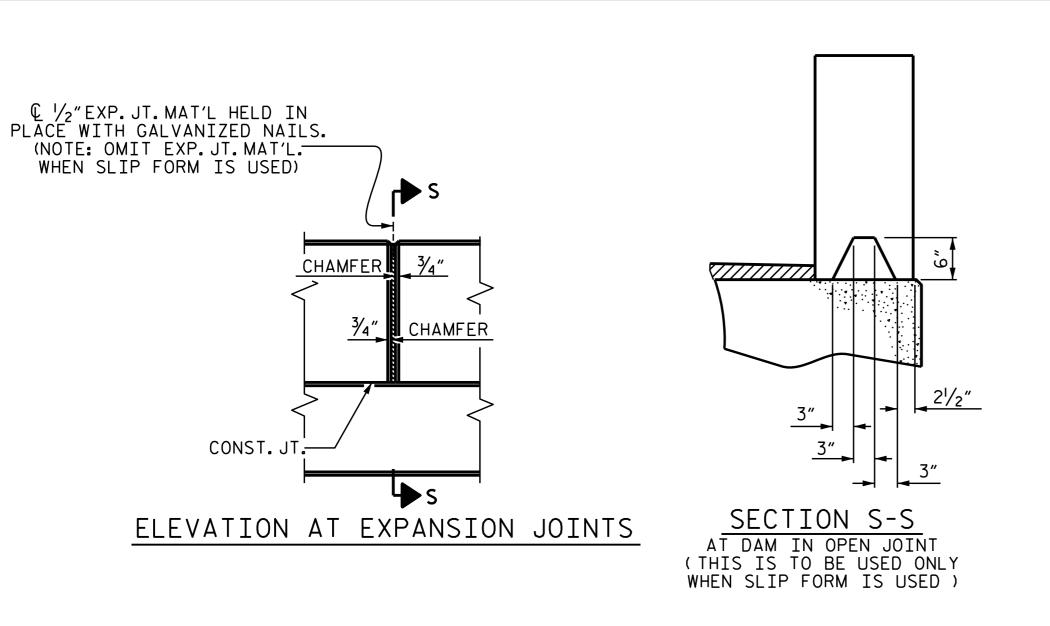
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

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Doddes died by
Douglas K. Callioun
12EFE12F0292440 11/21/2014

RALEIGH
STANDARD
3'-0" X 2'-0"
PRESTRESSED CONCRETE
CORED SLAB UNIT

	SHEET N				
BY:	DATE:	NO.	BY:	DATE:	S-6
		3			TOTAL SHEETS
		4			17

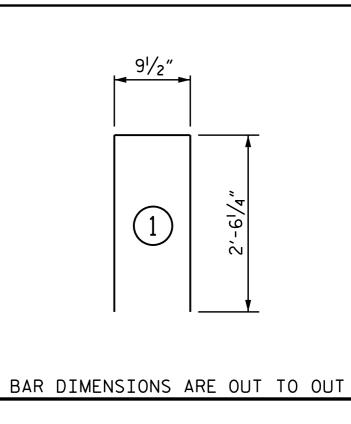
ASSEMBLED BY: A.C. OUTLAW DATE: 5/05/14 CHECKED BY: W.F. PARKER DATE: 5/08/14 DRAWN BY : MAA 6/10 REV. 12/11 CHECKED BY : MKT 7/10



NOTES

ALL REINFORCING STEEL IN PARAPETS AND END POSTS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, ½"IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



BAR TYPE

SPLICE LEN	IGTH CHART
BAR SIZE	EPOXY COATED
#5	3′-5″

2 PARAPETS & 4 END POSTS NO. | SIZE | TYPE | LENGTH | WEIGHT #5 STR 1368 *B25 13′-8″ #7 | STR 2'-10" 46 **#**7 STR 3'-4" 55 **∗**E3 **#**7 STR 3′-10″ 63 #7 STR 4'-4" 71 **∗** E5 #7 STR 4'-8" 76 8 #6 STR **∗** F1 1'-10" 22 STR 2'-11" #6 18 #6 STR 3'-7" 22 * F4 #6 STR 19 3'-1" 23 **⋇** F5 STR 3′-9″ 4 #6 * S6 122 #5 5′-10″ 742

BILL OF MATERIAL

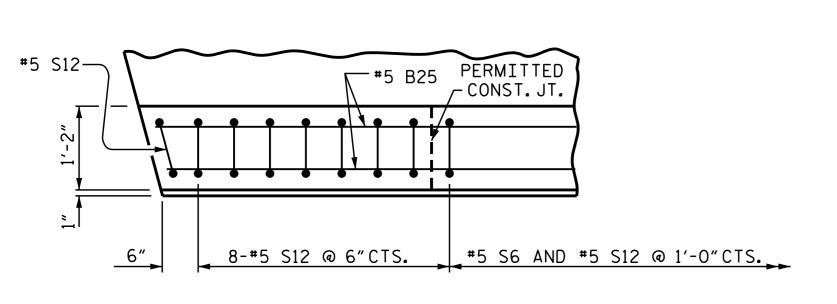
CLASS AA CONCRETE CU.YDS. 18.2

CONCRETE PARAPET LIN.FT. 140.0

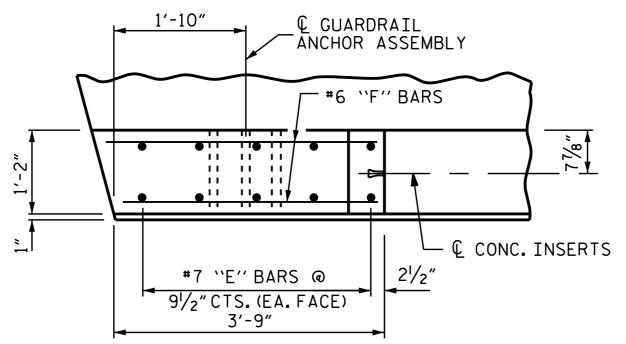
NOTE: #5 S12 BARS ARE INCLUDED IN THE BILL OF MATERIAL FOR CORED SLAB

UNITS.

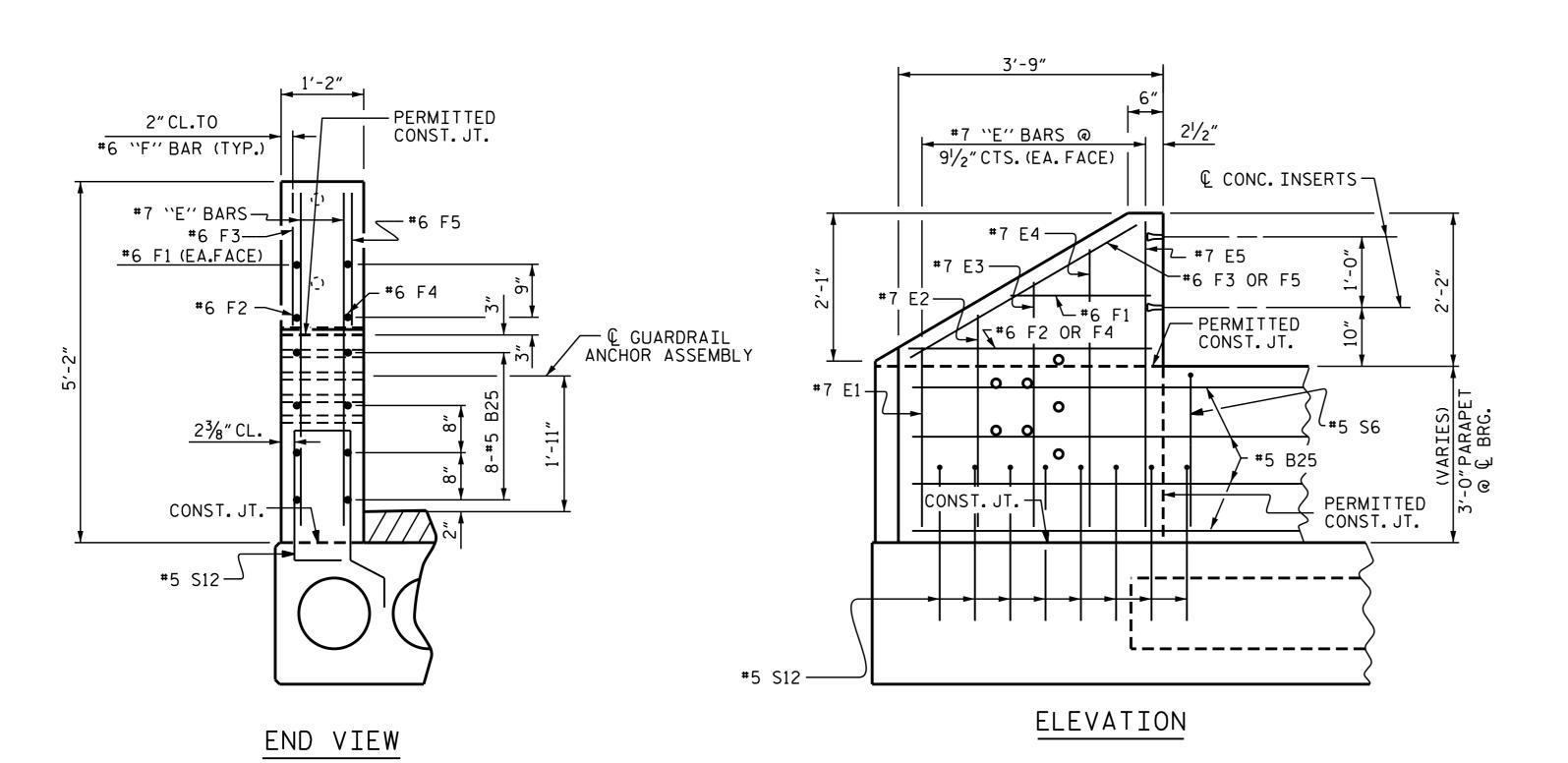
* EPOXY COATED REINFORCING STEEL LBS. 2525



PLAN OF PARAPET

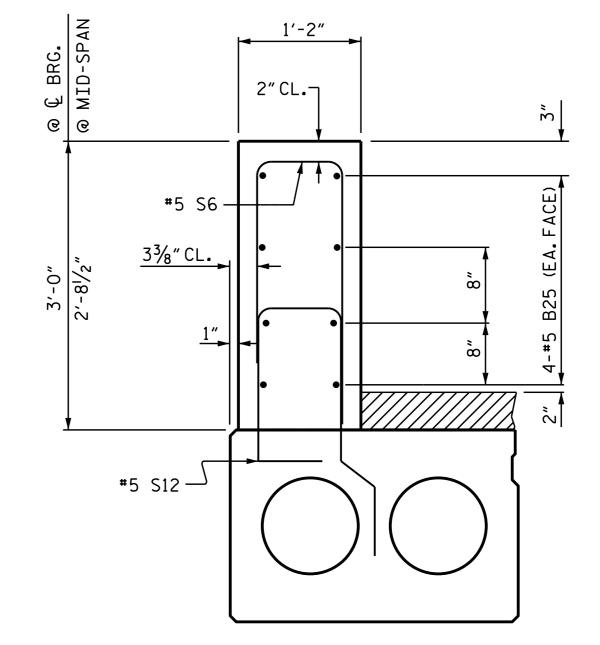


PLAN OF END POST



PARAPET AND END POST FOR TWO BAR METAL RAIL

DRAWN BY :	A.C. OUTLAW	DATE:5/05/14
CHECKED BY :	W.F. PARKER	DATE :5/08/14



SECTION THRU PARAPET

PROJECT NO. BD-5107Y

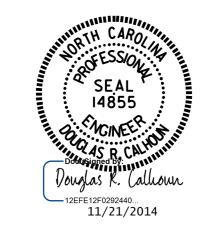
ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

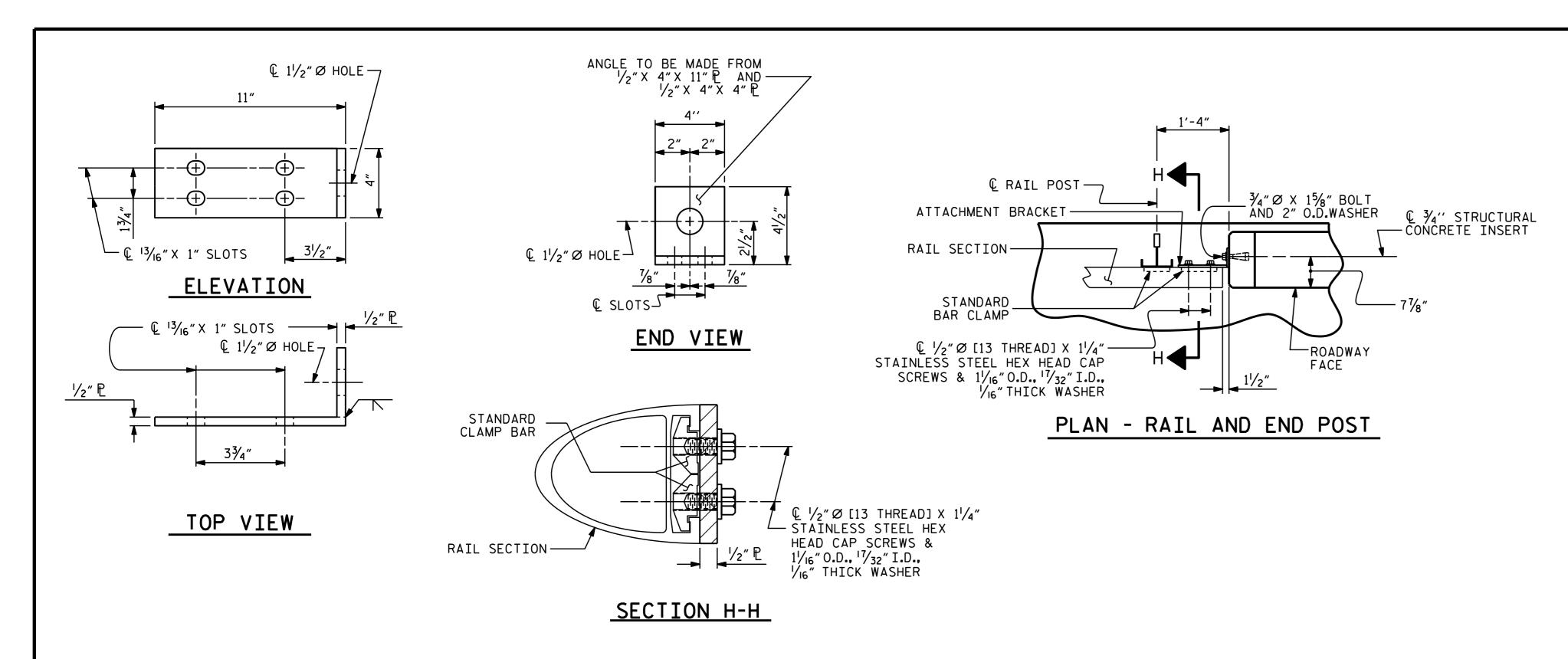
SHEET 4 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

CONCRETE PARAPETS AND END POSTS



	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-7
		3			TOTAL SHEETS
		4			17



DETAILS FOR ATTACHING METAL RAIL TO END POST

REV. 5/7/03 REV. 5/I/06

REV. 10/1/11

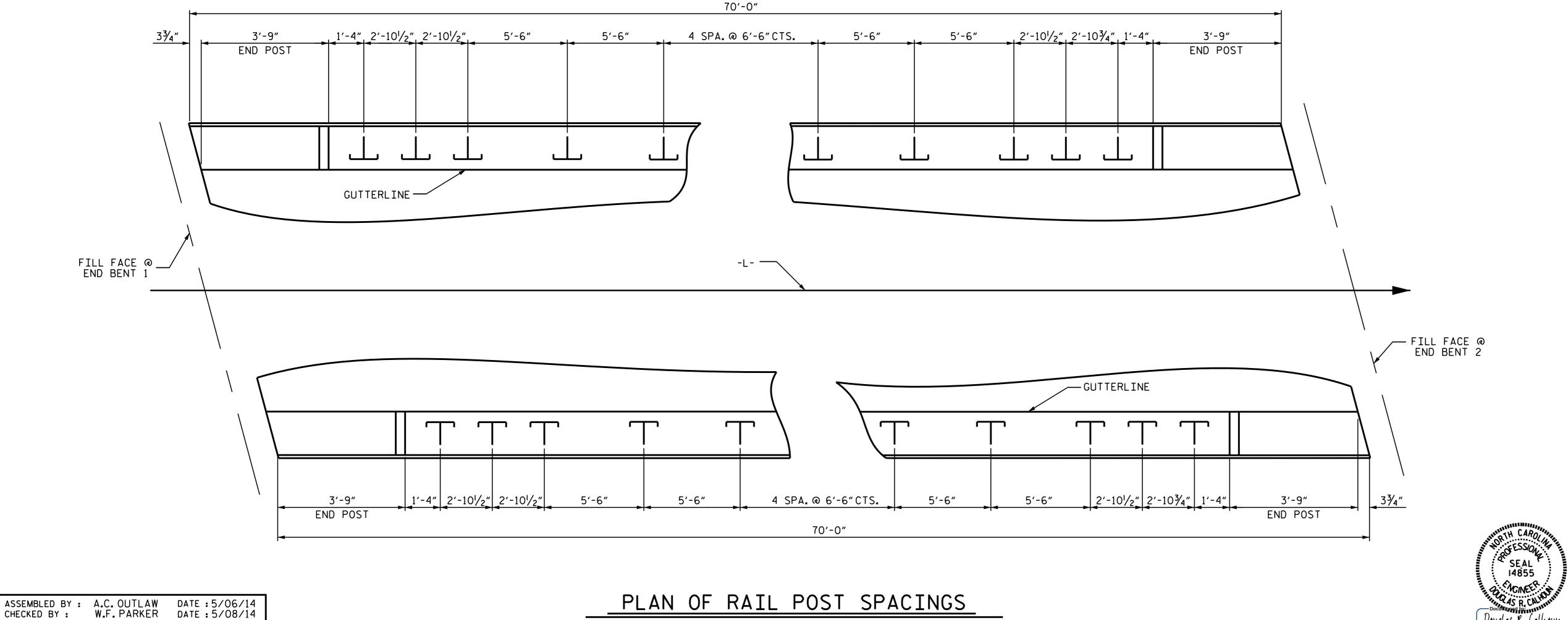
DRAWN BY: FCJ 1/88

CHECKED BY : CRK 3/89

RWW/JTE

MAA/GM

TLA/GM



NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 11/2".
- B. 1 $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A γ_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4" Ø X 15/8" BOLT WITH 2"O.D. WASHER IN PLACE. THE 3/4" Ø X 15/8" BOLT SHALL HAVE N. C. THREADS.

A. 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.

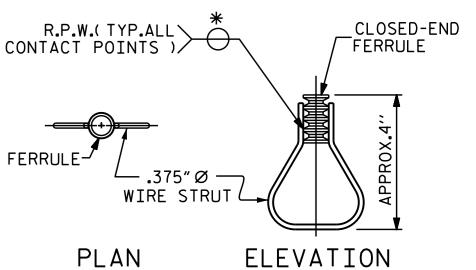
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. 1/2" Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" Ø X 15/8" BOLT WITH WASHER SHALL BE REPLACED WITH A 3/4" Ø X 61/2" BOLT AND 2"O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 34" Ø X 15%" BOLT SHALL APPLY TO THE 34" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



ELEVATION

STRUCTURAL CONCRETE

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

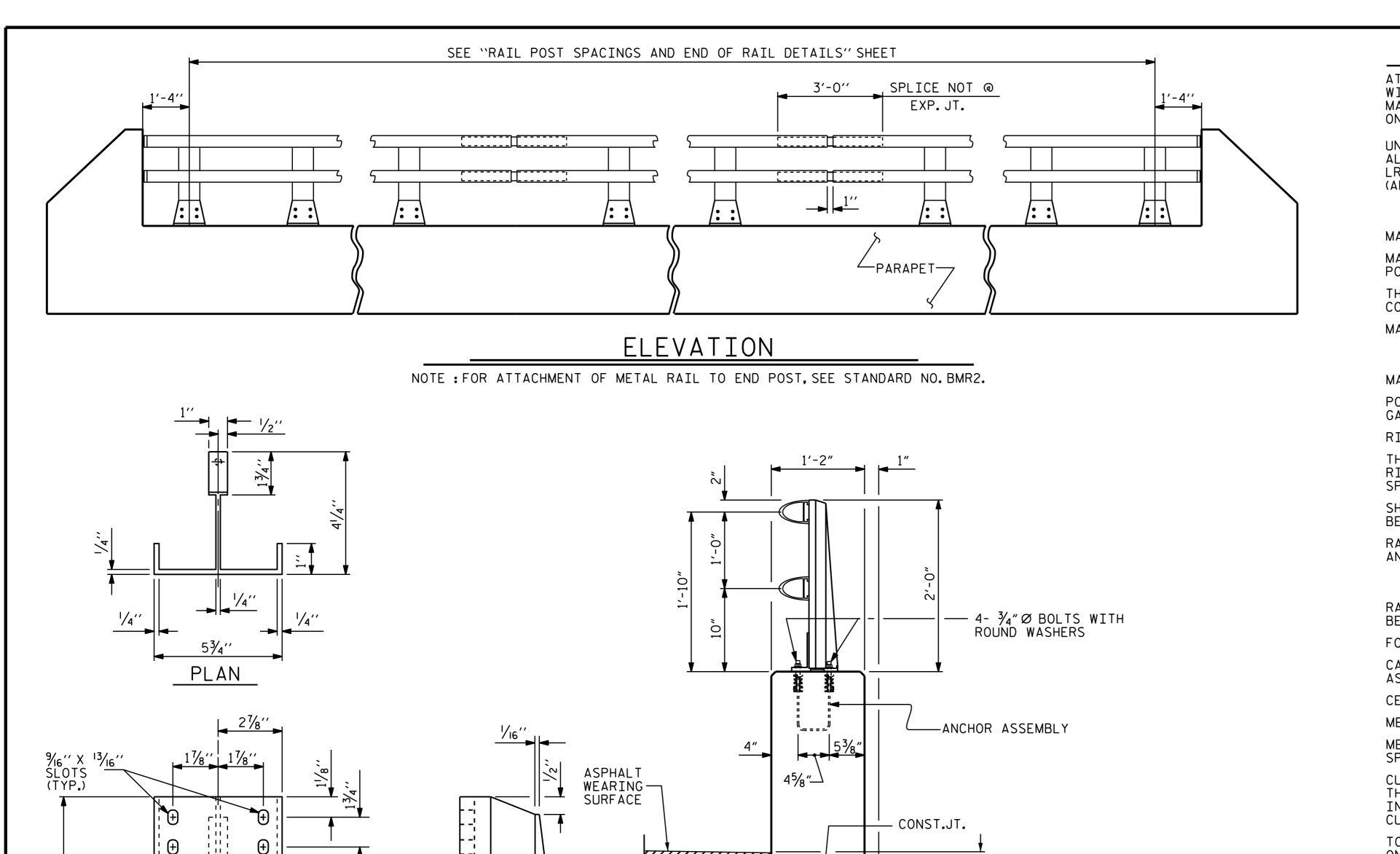
PROJECT NO. BD-5107Y ROCKINGHAM COUNTY STATION: 13+93.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

RAIL POST SPACINGS END OF RAIL DETAILS

REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:	S-8	
1			3			TOTA SHEET	
2			4			17	

PLAN OF RAIL POST SPACINGS



SECTION THRU

PARAPET AND RAIL

1 11 1

1 11 1

1 11 1

1 11 1

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1 11 1

FRONT ELEVATION

REV. 5/I/06 REV. IO/I/II REV. 6/I3

ASSEMBLED BY: A.C. OUTLAW DATE: 5/05/14 CHECKED BY: W.F. PARKER DATE: 5/08/14

4 - .766" Ø HOLES —

PUNCHED FOR RIVETS

DRAWN BY: EEM 6/94

CHECKED BY : RGW 6/94

 \oplus

 \oplus

5/₁₆ " Ø DRILL 1" DEEP & 3/₈" Ø [16 THREAD] TAP

₩ 3" DEEP FOR 3%" Ø X 1 1/2"

MAA/GM

MAA/GM

STAINLESS STEEL CAP SCREW

DETAILS OF POST

SIDE ELEVATION

NOTES

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE OPTION TO USE AN ALTERNATE TO THE 2 BAR METAL RAIL. THE ALTERNATE RAIL SHALL MEET THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MUST BE LISTED ON THE DEPARTMENT'S APPROVED PRODUCTS LIST (APL) UNDER "2 BAR METAL RAIL ALTERNATE". ADJUSTMENTS TO THE CONCRETE PARAPET WILL NOT BE ALLOWED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6.

MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

SEAL 14855

MICINEER

Douglas K. Calhoun

11/21/2014

PAY LENGTH = $\frac{124.38 \text{ LIN. FT.}}{}$

(;;500;±);750;745;7 RIVET DETAIL

DRILL & COUNTER BORE

FOR 3/8" Ø [16 THREAD] CAP SCREW PROJECT NO. BD-5107Y

ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

2 BAR METAL RAIL

REVISIONS SHEET NO. NO. BY: DATE: S - 9 1 3 TOTAL SHEETS 2 4 17							
1 3 TOTAL SHEETS							
	NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
2 4 17	1			3			TOTAL SHEETS
	2			4			17

FRONT ELEVATION

SIDE ELEVATION

PERMITTED WELD

SIDE ELEVATION

POST BASE DETAILS

4 - .766′′Ø

HOLES PUNCHED FOR RIVETS

VARIES

3¾′′

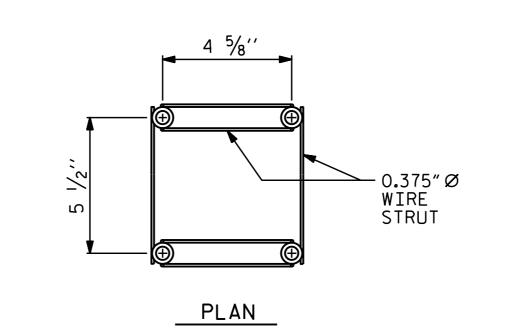
 45/8′′

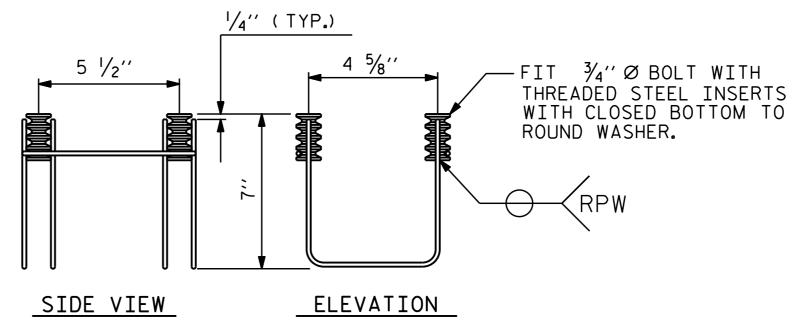
PLAN

NOTE : BASE CAN BE SUPPLIED

EXTRUSIONS WELDED TOGETHER

AS ONE EXTRUSION OR TWO





METAL RAIL ANCHOR ASSEMBLY

(26 ASSEMBLIES REQUIRED

NOTES

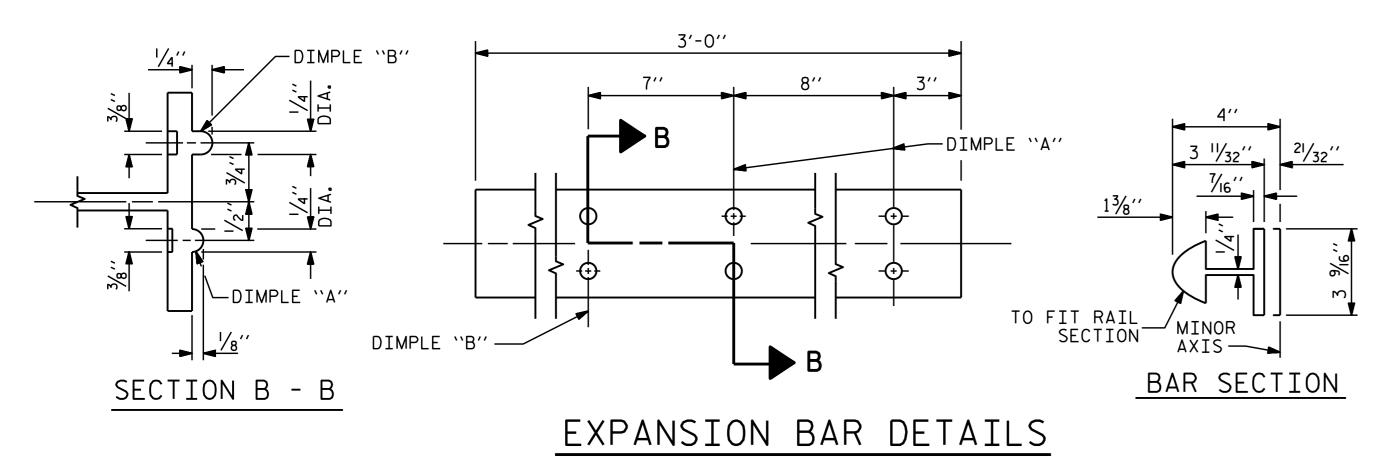
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

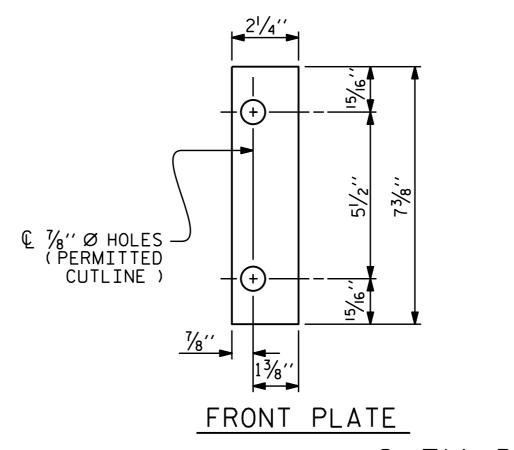
THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

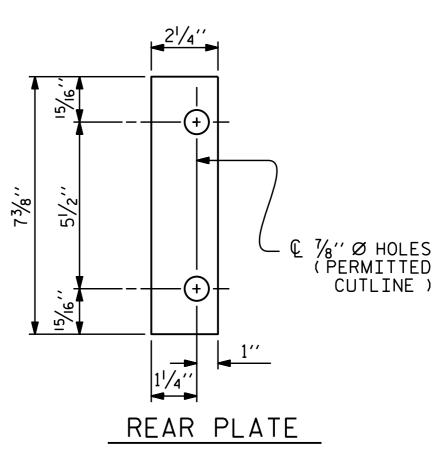
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR $\frac{3}{4}$ " FERRULES.
- B. 4 $\frac{3}{4}$ " Ø X 2 $\frac{1}{2}$ " BOLTS WITH WASHERS.BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $2\frac{1}{2}$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_{16} $^{\prime\prime}$ \varnothing WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

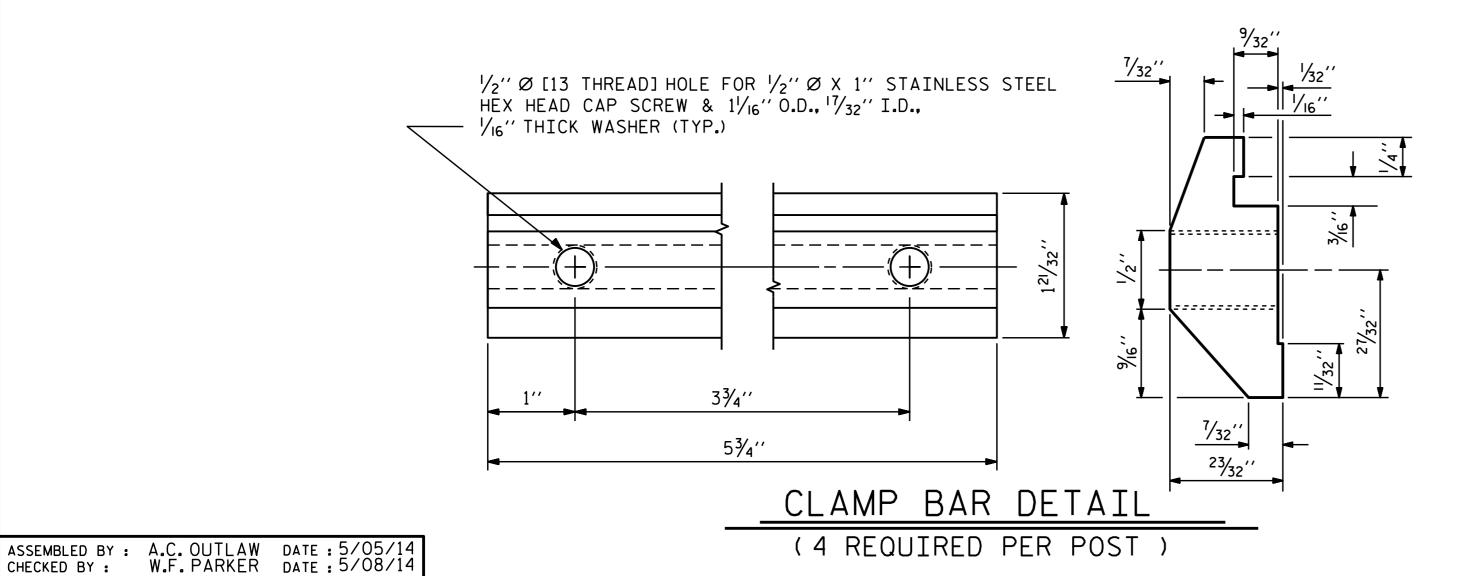


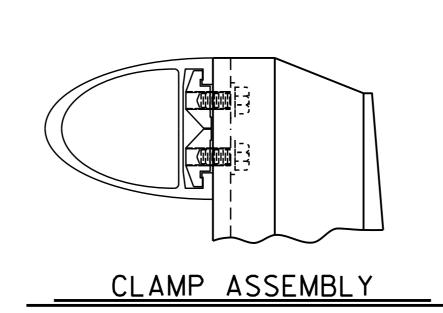


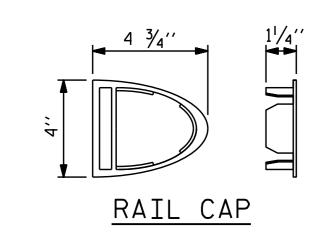


SHIM DETAILS

SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.









RAIL SECTION PROJECT NO. BD-5107Y ROCKINGHAM COUNTY STATION: 13+93.00 -L-

MINOR
 AXIS

4 3/4"

- SEMI-ELLIPSE

AXIS

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

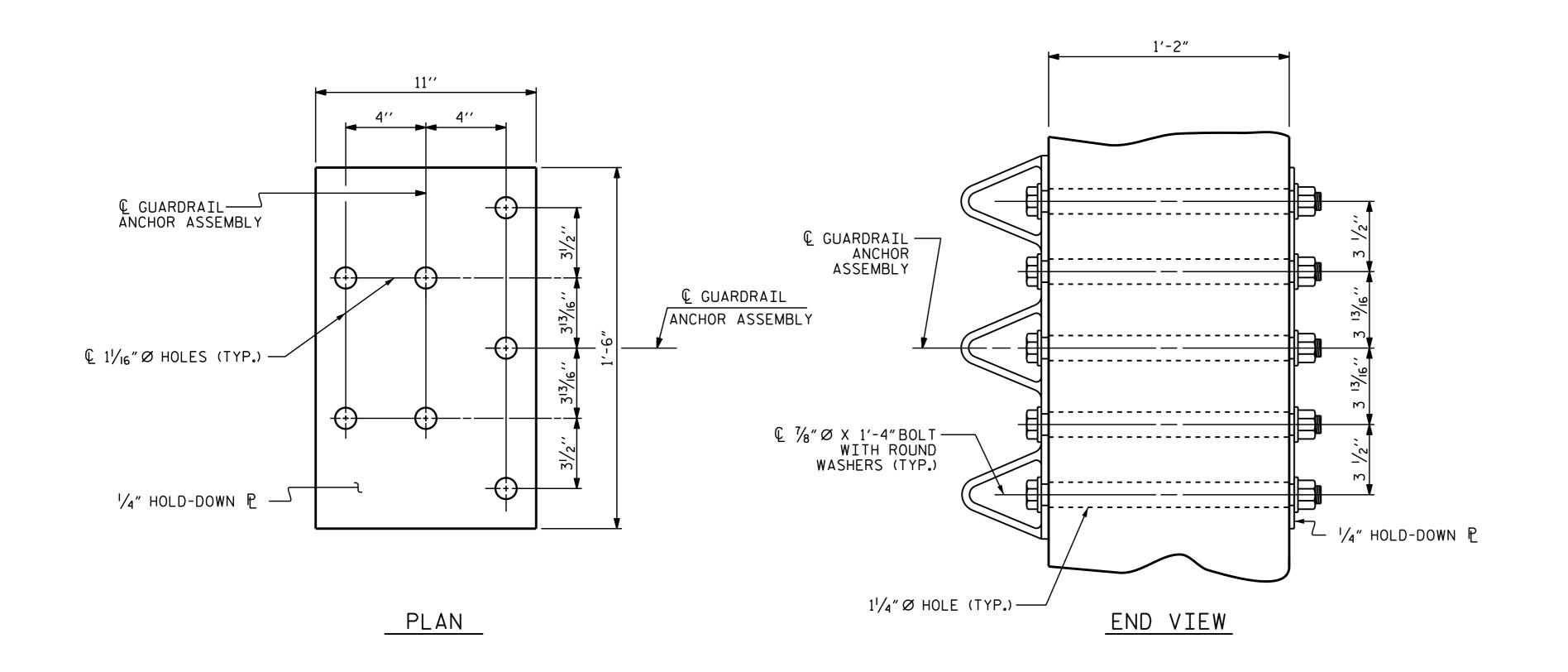
STANDARD

2 BAR METAL RAIL

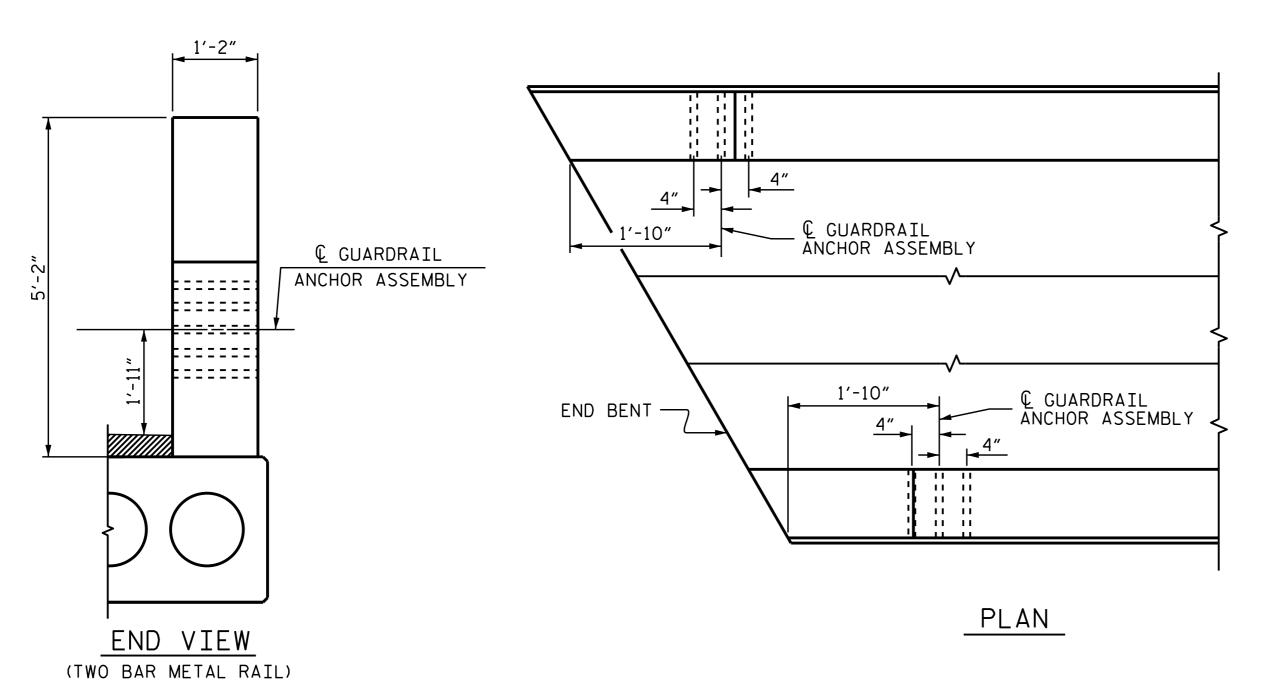
	SHEET N					
0.	BY:	DATE:	NO.	BY:	DATE:	S-10
0			3			TOTAL SHEETS
2			4			17

DRAWN BY: EEM 6/94 REV. 8/16/99 MAB/LES REV. 5/1/06R KMM/GM REV. 10/1/II MAA/GM

11/21/2014



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

ASSEMBLED BY: A.C. OUTLAW DATE: 5/06/14 CHECKED BY: W.F. PARKER DATE: 5/08/14

DRAWN BY: MAA 5/IO REV. IO/I/II REV. I2/5/II REV. 6/I3 MAA/GM MAA/GM

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE ½" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

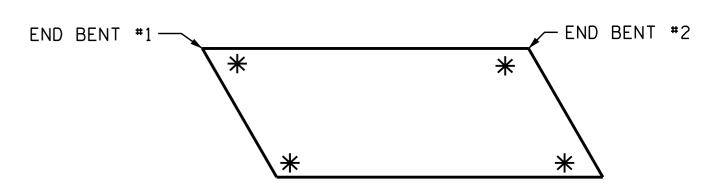
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



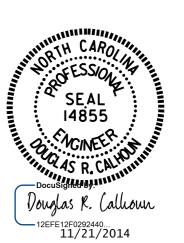
SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. BD-5107Y

ROCKINGHAM COUNTY

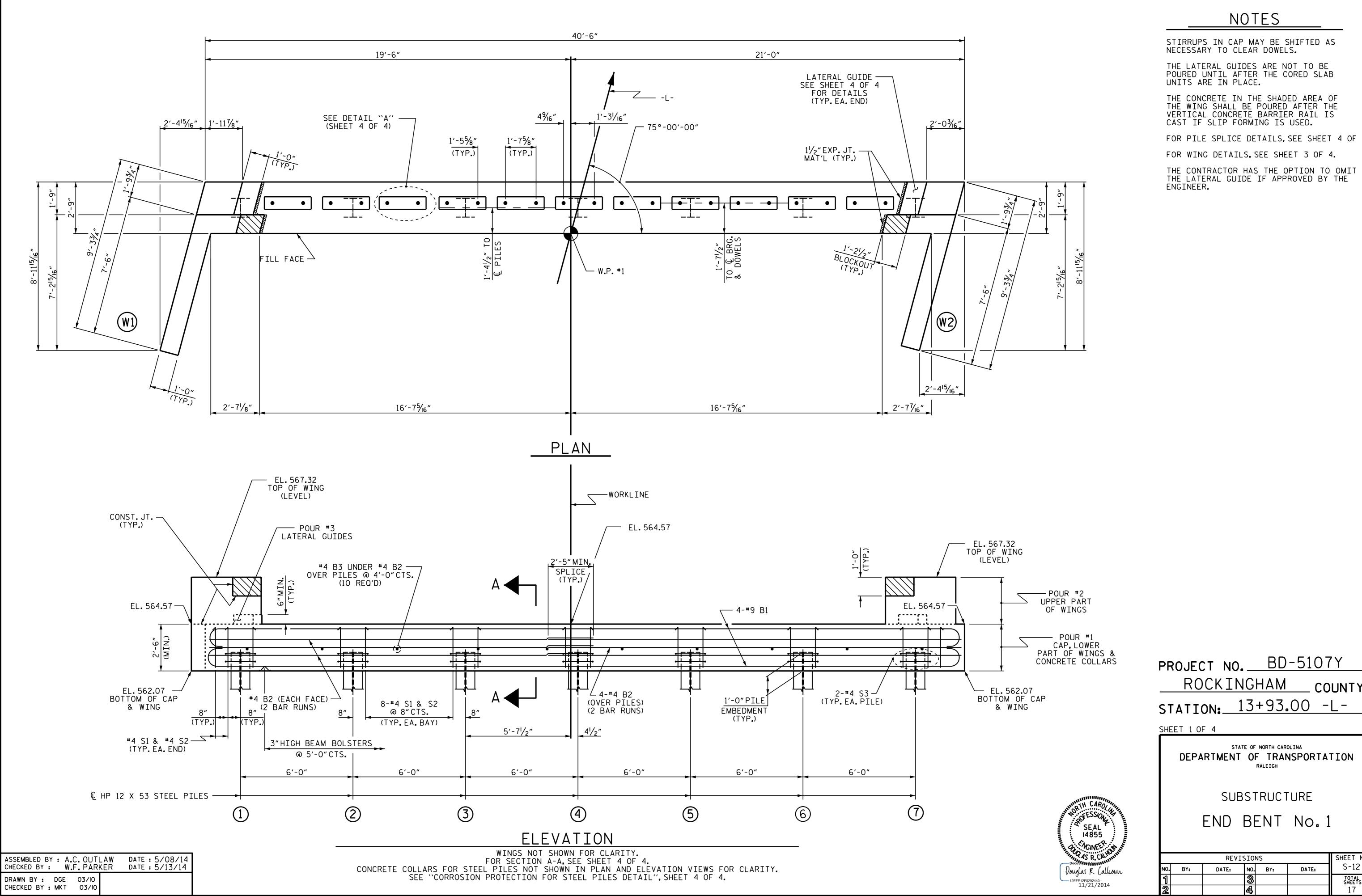
STATION: 13+93.00 -L-



DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

GUARDRAIL ANCHORAGE
DETAILS
FOR METAL RAILS

	REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:	S-11		
1			3			TOTAL SHEET		
2			4			17		



STD. NO. EB_33_75S

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE

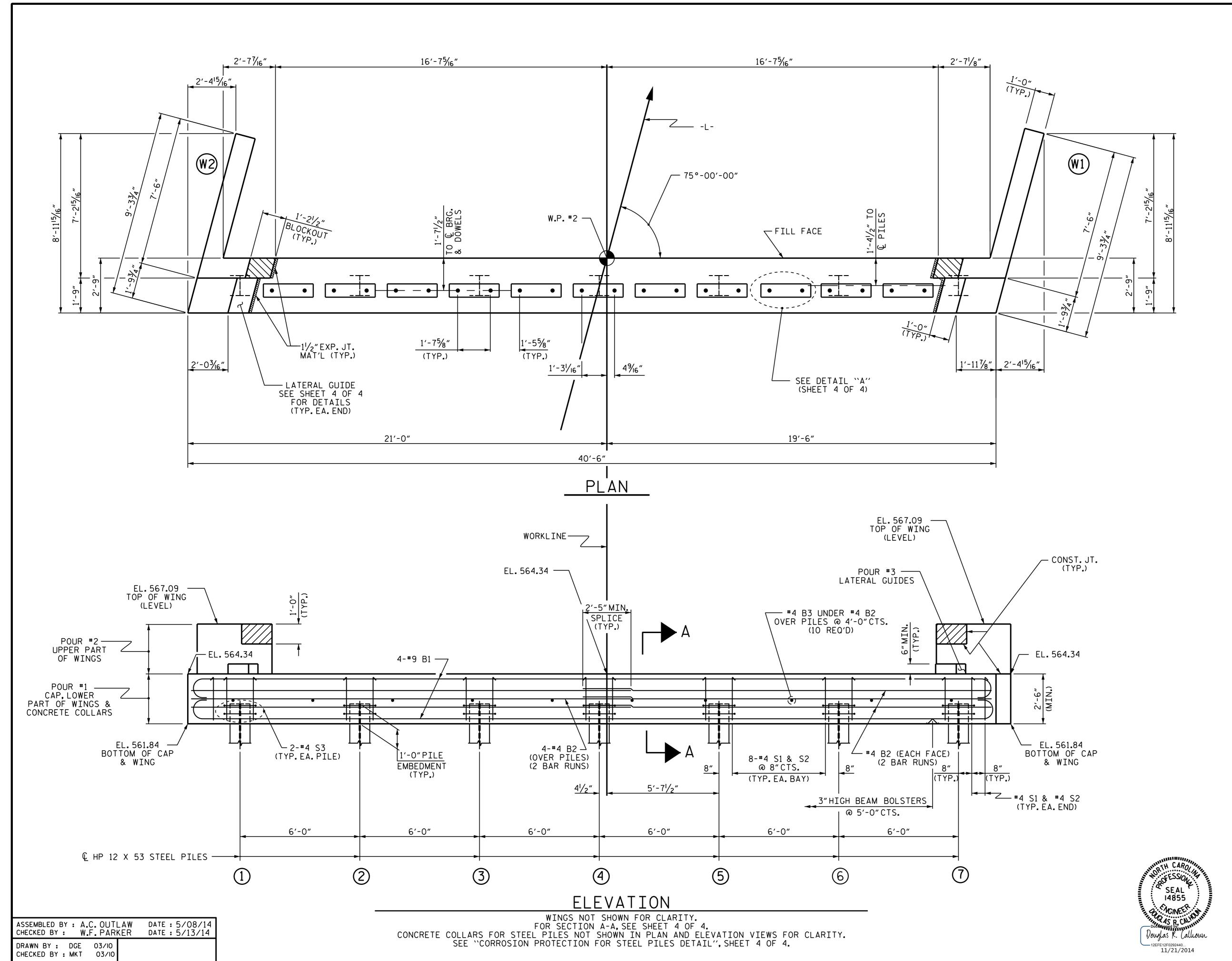
PROJECT NO. BD-5107Y ROCKINGHAM __ COUNTY

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> > SUBSTRUCTURE

END BENT No. 1

	SHEET NO					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS
2			<u>a</u>			17



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

PROJECT NO. BD-5107Y

ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

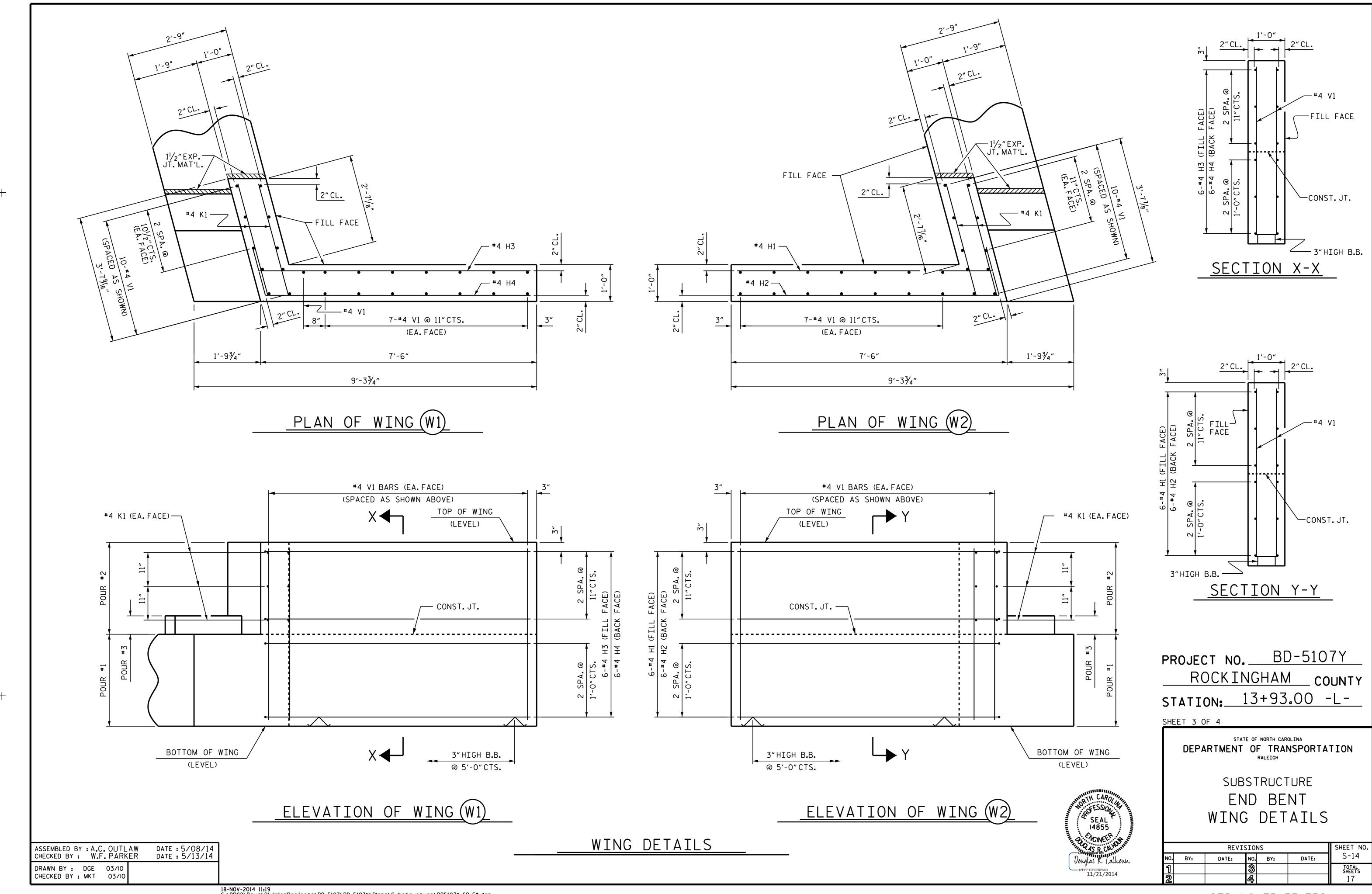
SHEET 2 OF 4

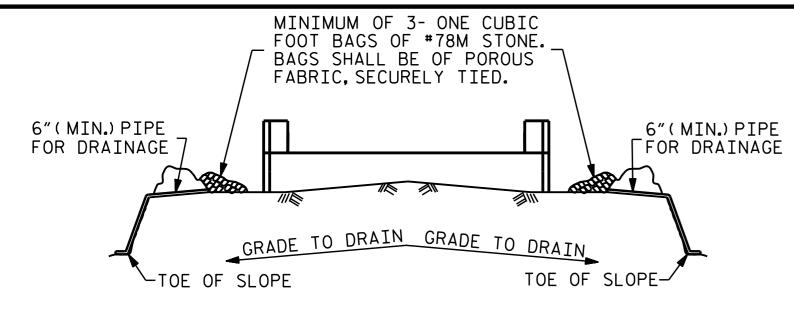
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 2

		SHEET NO.				
•	BY:	DATE:	NO.	BY:	DATE:	S-13
			3			TOTAL SHEETS
			N			17



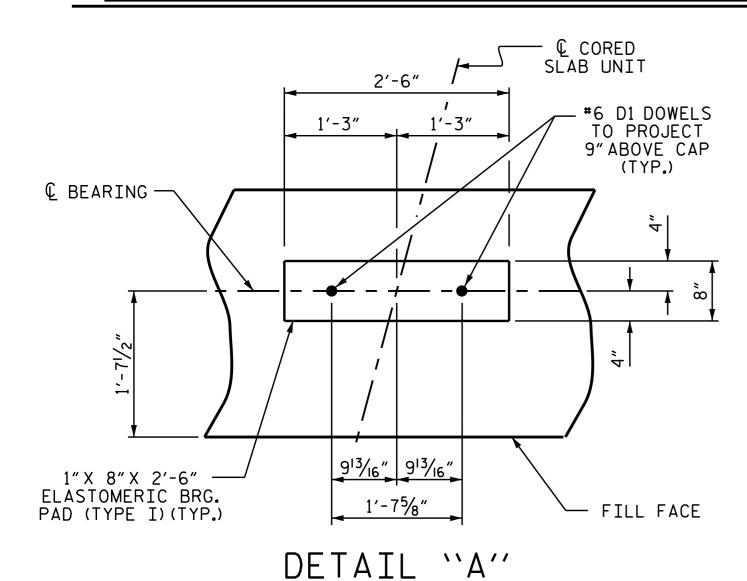


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

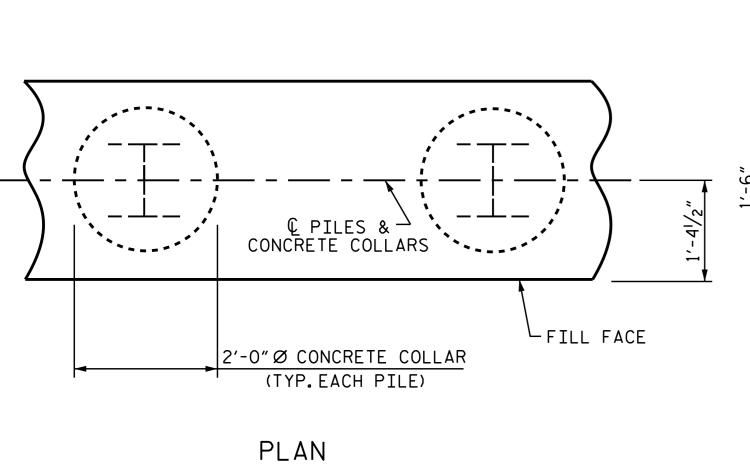
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

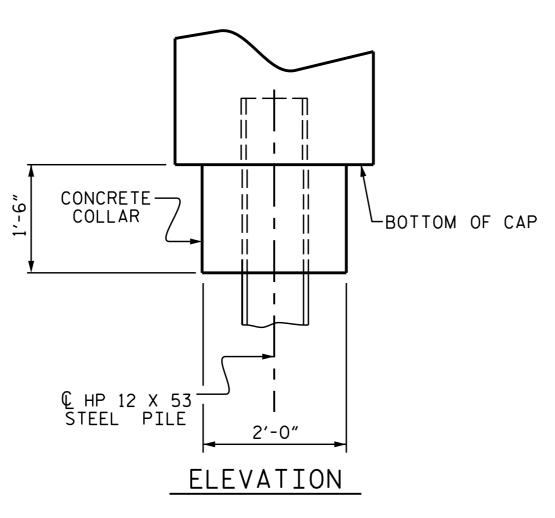
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

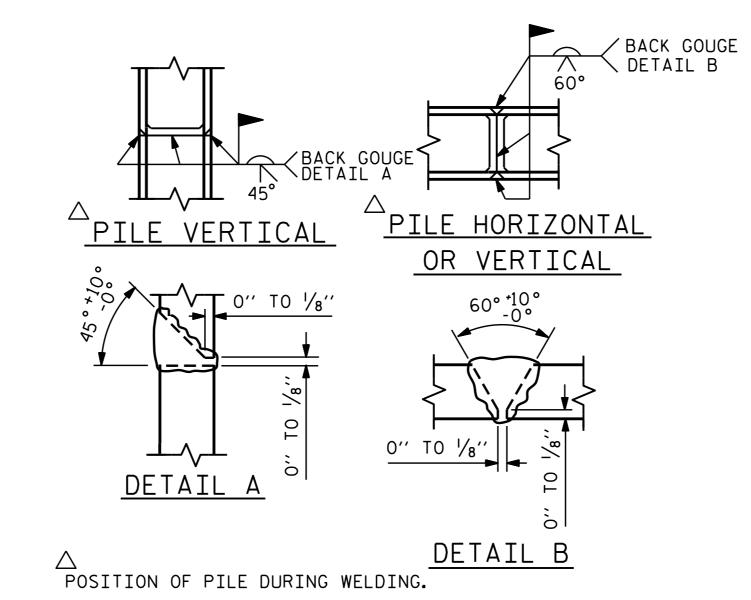




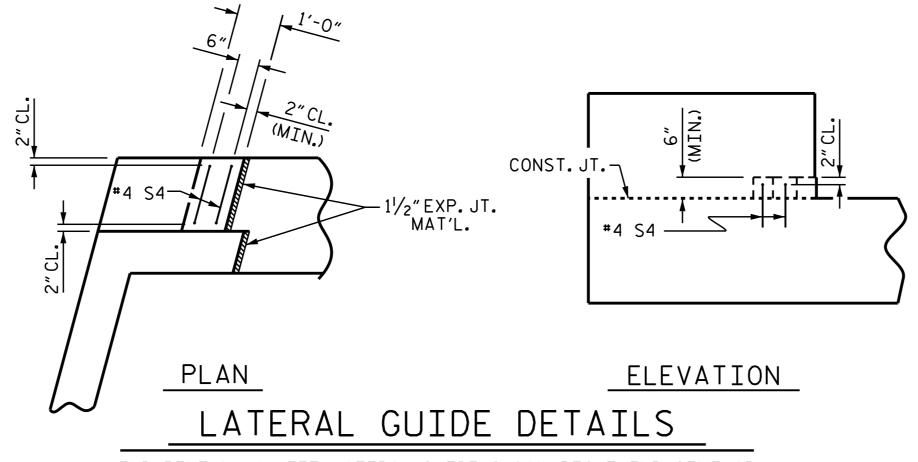
CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

ASSEMBLED BY : A.C. OUTLAW DATE : 5/08/14 CHECKED BY : W.F. PARKER DATE : 5/13/14

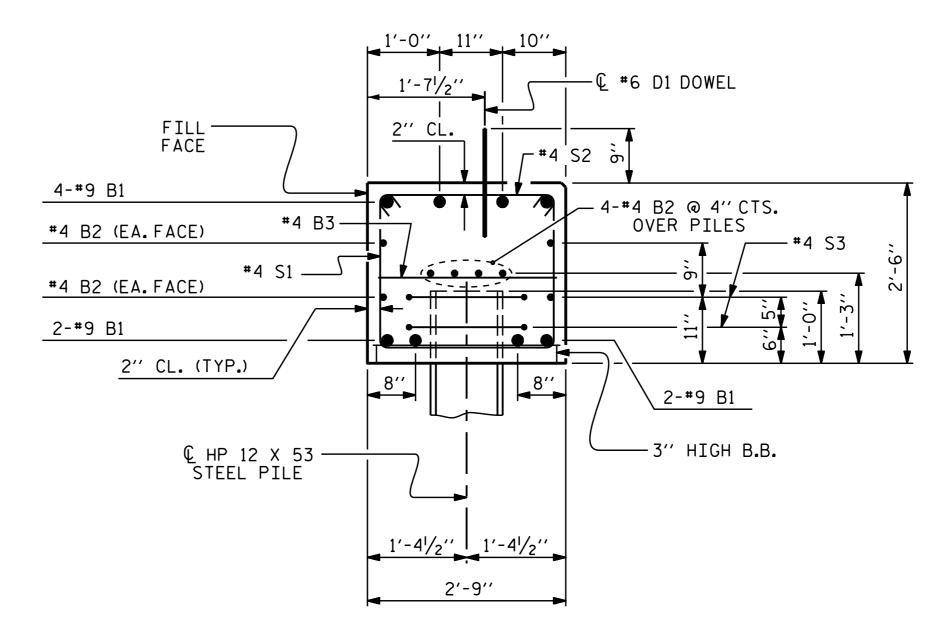
DRAWN BY : DGE 03/10 CHECKED BY : MKT 03/10



PILE SPLICE DETAILS

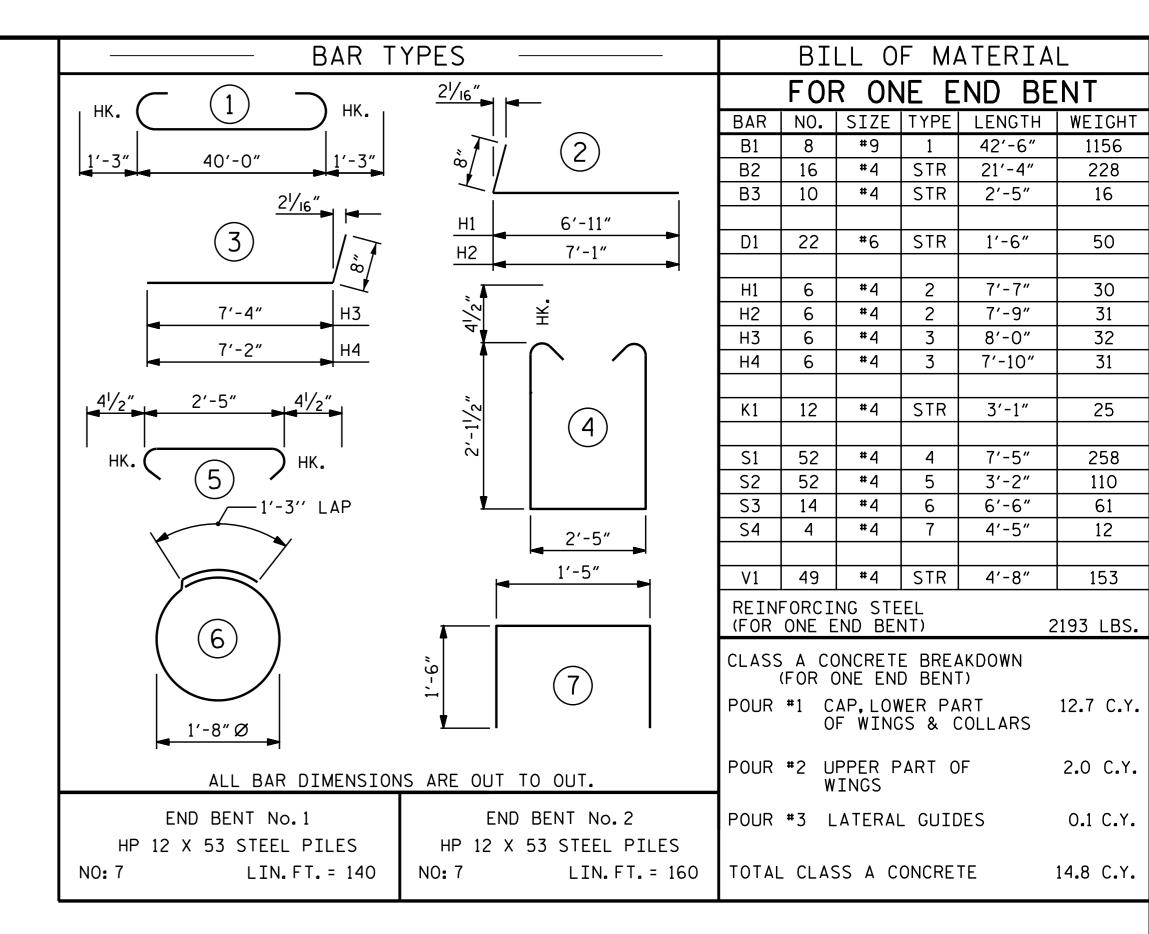


(END BENT No.1, LEFT LATERAL GUIDE SHOWN, RIGHT END SIMILAR)
(END BENT No.2 SIMILAR BY ROTATION)



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY.
SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")



PROJECT NO. BD-5107Y

ROCKINGHAM COUNTY

STATION: 13+93.00 -L-

SHEET 4 OF 4

SEAL 14855

GONEER OF ALLON

-12EFE12F0292440... 11/21/2014 STATE OF NORTH CAROLINA

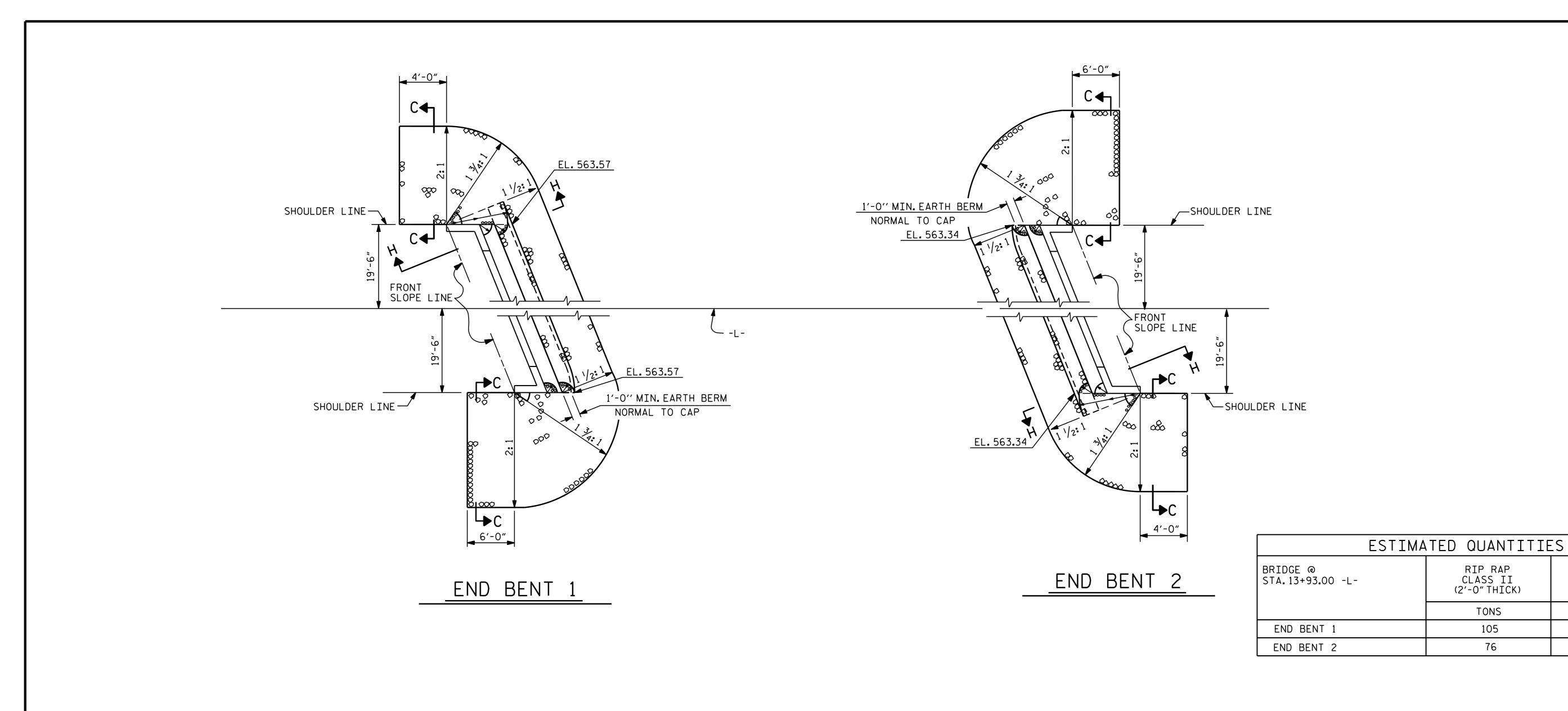
DEPARTMENT OF TRANSPORTATION

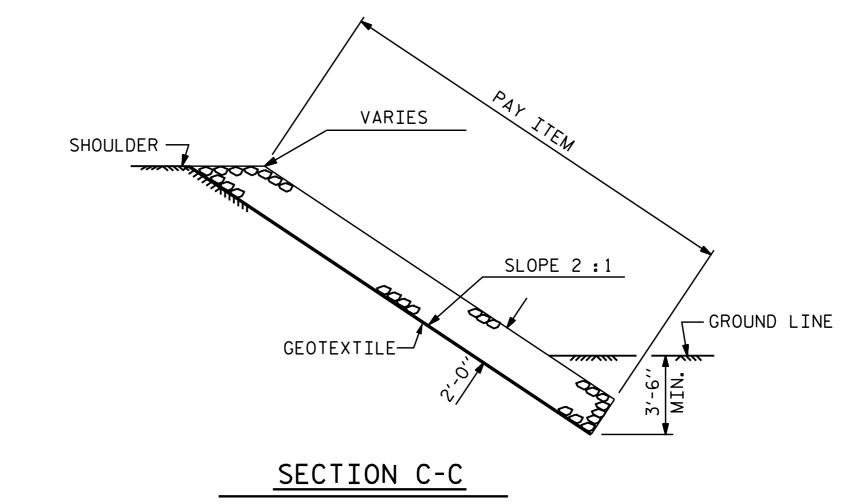
RALEIGH

SUBSTRUCTURE

END BENT No.1 & 2
DETAILS

	SHEET NO					
NO. BY	:	DATE:	NO.	BY:	DATE:	S-15
1			®			TOTAL SHEETS
2			4			17





PROJECT NO. BD-5107Y

ROCKINGHAM county

STATION: 13+93.00 -L-

GEOTEXTILE FOR DRAINAGE

SQUARE YARDS

116

85

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

C. T. A. N.D. A. D.D.

STANDARD

---RIP RAP DETAILS---

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-16

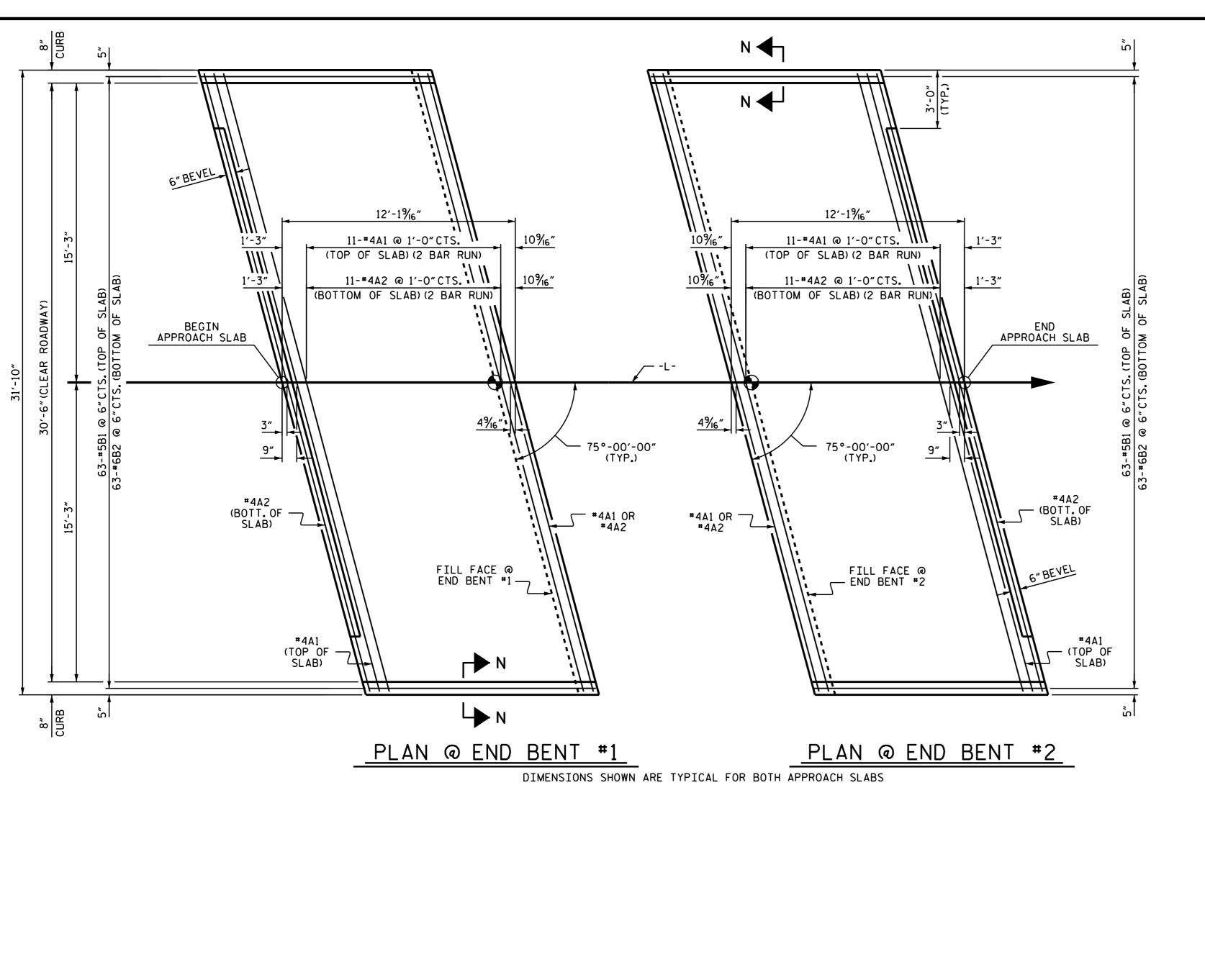
1 3 5 17

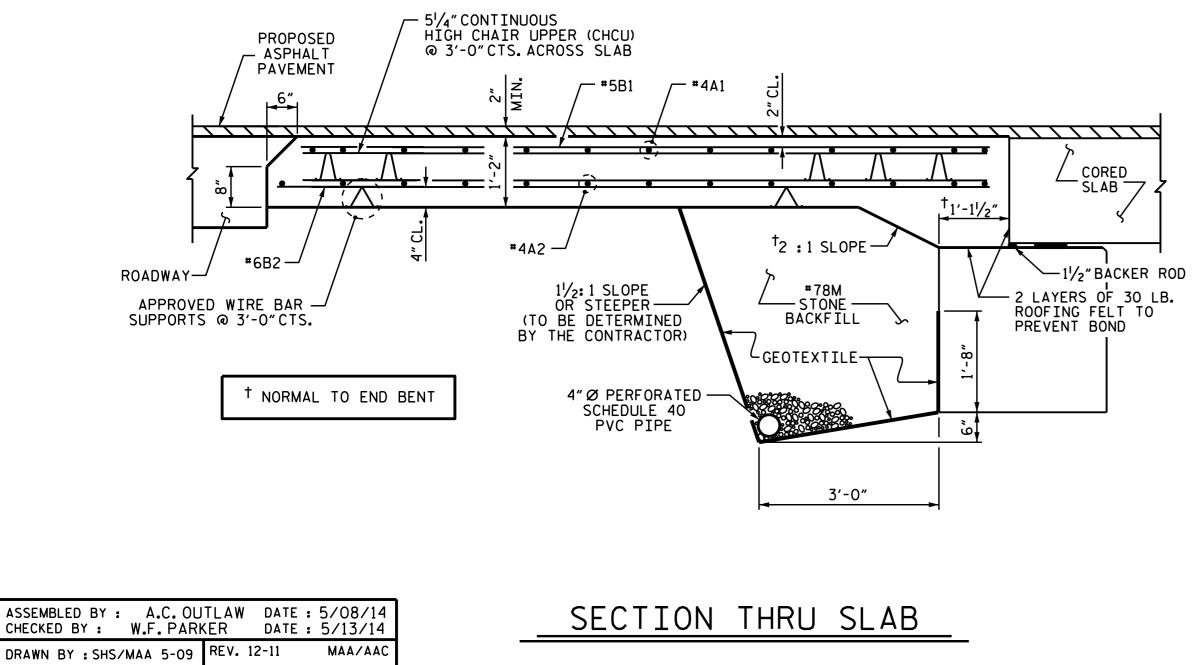
1'-7" MIN. BERM NORMAL TO CAP 1'-7" MIN. BERM NORMAL TO CAP SHOULDER LINE -EL. 563.57 (END BENT 1) EL. 563.34 (END BENT 2) EL. 563.57 (END BENT 1) SLOPE 1 1/2:1 EL. 563.34 (END BENT 2) SLOPE 1 1/2:1 - GROUND LINE GROUND LINE 1'-0" MIN. EARTH BERM NORMAL TO CAP GEOTEXTILE-1'-0'' MIN. EARTH BERM
NORMAL TO CAP GEOTEXTILE-SECTION H-H BERM RIP RAPPED

ASSEMBLED BY: A.C. OUTLAW DATE:5/08/14 CHECKED BY: W.F. PARKER DATE:5/13/14

DRAWN BY: REK 1/84 REV. 8/16/99 REV. 10/17/00 REV. 5/1/06R RWW/LES REV. 5/1/06R TLA/GM

11/21/2014





CHECKED BY : BCH 5-09

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

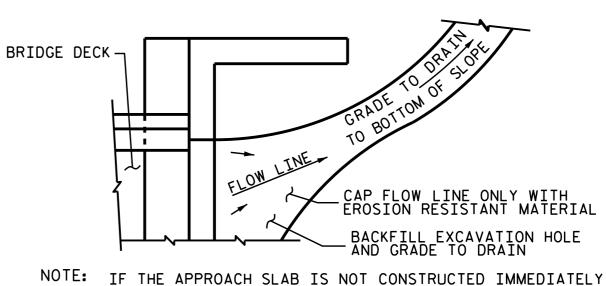
*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

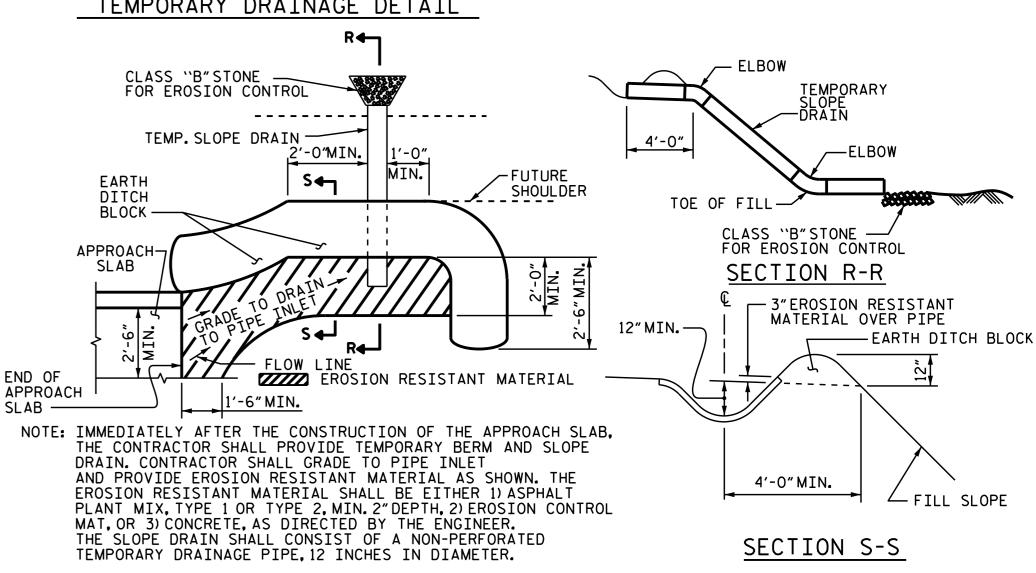
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE.
THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE
MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

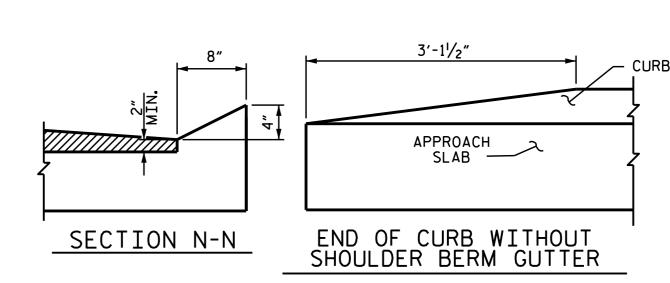
TEMPORARY DRAINAGE DETAIL



PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



CURB DETAILS

			_					
PLICE LENGTHS								
AR [ZE	EPOXY COATED	UNCOATED						
4	2'-0"	1'-9"						
5	2'-6"	2'-2"						
6	3′-10″	2'-7"						

11/21/2014 11/21/2014

PROJECT NO. BD-5107Y ROCKINGHAM COUNTY STATION: 13+93.00 -L-

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR NO. SIZE TYPE LENGTH WEIGHT

300

728

1096

1396

1029

19.9

301

300

728

1096

1396

1029

C. Y. 19.9

LBS.

LBS.

C.Y.

LBS.

LBS.

26 | #4 | STR | 17'-4"

26 | #4 | STR | 17'-3"

63 #6 | STR | 11'-7"

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

26 | #4 | STR | 17'-4"

26 #4 STR 17'-3"

63 #6 STR 11'-7"

*B1 | 63 | #5 | STR | 11'-1"

*B1 | 63 | #5 | STR | 11'-1"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

***** EPOXY COATED

REINFORCING STEEL

***** EPOXY COATED

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE

CORED SLAB UNIT (SUB-REGIONAL TIER) 75° SKEW

				•	
	SHEET N				
BY:	DATE:	NO.	BY:	DATE:	S-17
		3			TOTAL SHEETS
		4			17

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ----- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. ---- 1,200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS ---- 1.800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER - - - -EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH